

Shear, pure and simple

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Abstract

In a 2012 article in the International Journal of Non-Linear Mechanics, Destrade et al. showed that for nonlinear elastic materials satisfying Truesdell's so-called empirical inequalities, the deformation corresponding to a Cauchy pure shear stress is not a simple shear. We confirm their results under weakened assumptions and consider the case of a shear load, i.e. a Biot pure shear stress. In addition, conditions under which Cauchy pure shear stresses correspond to (idealized) pure shear stretch tensors are stated and a new notion of idealized *finite simple shear* is introduced, showing that for certain classes of nonlinear materials, the results by Destrade et al. can be simplified considerably.

Key words: nonlinear elasticity, finite isotropic elasticity, constitutive inequalities, stress tensors, Cauchy stress, isotropic nonlinear elasticity, isotropic tensor functions, stress increases with strain, constitutive law, conjugate stress-strain pairs, hyperelasticity, Cauchy-elasticity, empirical inequalities, adscitious inequalities, Poynting-effect, Kelvin-effect

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1 Introduction

The term “shear” describes a number of closely related, but distinct concepts which play an important role in linear and nonlinear elasticity theory. While the notion of a (*pure*) *shear stress* $T = s(e_1 \otimes e_2 + e_2 \otimes e_1)$ with $s \in \mathbb{R}$ is rather straightforward [4, 5] (once the stress tensor is specified), the concept of a *shear deformation* is rather ambiguous: typically, the term (*simple*) *shear (deformation)* is used to describe a specific homogeneous deformation gradient $\nabla\varphi = F \in \text{GL}^+(3)$ of the form $F = \mathbb{1} + \gamma e_1 \otimes e_2$ with the amount of shear $\gamma \in \mathbb{R}$. On the other hand, a stretch tensor $P \in \text{Sym}^+(3)$ is often called a (*pure*) *shear (stretch)* if the principal stretches $\lambda_1, \lambda_2, \lambda_3$ are of the form $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$ and $\lambda_3 = 1$ with $\lambda > 0$, see e.g. [12, 19, 26]. Hence, a pure shear stretch is a three-dimensional homogeneous deformation of a body which is elongated in one direction while being shortened in equal ratio perpendicularly.

The latter requirement is satisfied by the stretch corresponding to any simple shear deformation F with $\lambda = \frac{1}{2}(\sqrt{\gamma^2 + 4} + \gamma)$. Since any pure shear stretch corresponds to a simple shear F by appropriate left- and right-hand rotation, these two notions of shear often get confounded in the literature. In a 2012 article, Destrade et al. [6] clarified the relation between Cauchy pure shear stresses and simple shear deformations, demonstrating their incompatibility in nonlinear elasticity. Similar observations can be found in an article by Moon and Truesdell [18] and a more current paper by Mihai and Goriely [15]. In the following, we extend their discussion on the notion of shear in nonlinear elasticity. Undoubtedly, the primary concept in nonlinear elasticity is the force (stress) which causes the deformation. Therefore, our answer as to what a shear deformation is starts from considering Cauchy pure shear stress tensors.

1.1 Overview

After a short introduction and a brief discussion of the linear case in Section 1.3, we demonstrate in Section 2 that non-trivial Cauchy pure shear stress tensors never correspond to simple shear deformations for arbitrary non-linear isotropic elasticity laws. This result was previously obtained by Destrade et al. [6] in the special case of constitutive laws satisfying Truesdell’s so-called *empirical inequalities*. We then provide a number of representations for the class of deformations which do correspond to Cauchy pure shear stresses, including the composition of a triaxial stretch with a simple shear deformation given by Destrade et al. [6], cf. [18, 15]. In Appendix A, we extend these considerations to the case of Biot pure shear stress tensors and show that analogous results hold.

Based on this classification, we introduce constitutive requirements for hyperelastic laws which ensure that the principal stretches $\lambda_1, \lambda_2, \lambda_3$ corresponding to pure shear stresses are of the form $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$ and $\lambda_3 = 1$ and provide a representation of deformation gradients with this particular property in Sections 3 and 4.

While the results we obtain in Section 2 are not new [18, 6, 15], we hope that by following a deliberately careful and detailed approach, we are able to clear up any confusion the reader might still face regarding the general relation between shear deformations and shear stresses in nonlinear elasticity. We also try to highlight the differences between the proofs given in [18], [6] and [15], especially with respect to their additional requirements on the constitutive law, cf. Remark 2.6 and Remark 2.11.

1.2 Different notions of shear

The classical (homogeneous) *simple shear deformation* $F = \mathbb{1} + \gamma e_2 \otimes e_1$ of a unit cube with the amount of shear $\gamma \in \mathbb{R}^+ = (0, \infty)$ is shown in Figure 1. It is well known that in isotropic linear elasticity, the Cauchy stress tensor corresponding to deformations of this type is necessarily of the form $\sigma = s(e_1 \otimes e_2 + e_2 \otimes e_1)$ with $s \in \mathbb{R}$, known as *pure shear stress*, as visualized in Figure 2.

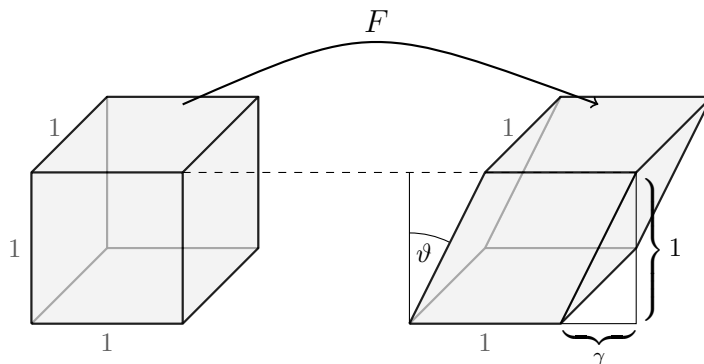


Figure 1: Simple shear deformation with shear angle ϑ and amount of shear $\gamma = \tan(\vartheta)$.

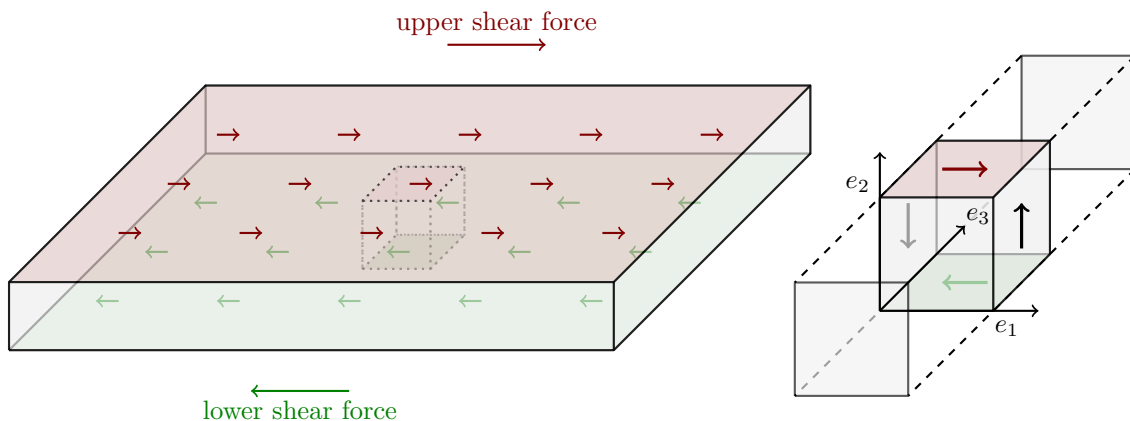


Figure 2: Visualization of pure shear stress. Right, Cauchy cube under pure shear stress in the current deformation.

Definitions 1.1. We call $F \in \text{GL}^+(3)$ a *simple shear deformation* if F has the form

$$F_\gamma = \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} = \mathbb{1} + \gamma e_2 \otimes e_1 \quad \text{with } \gamma \in \mathbb{R} \text{ the amount of shear} \quad (1.1)$$

and we call $T \in \text{Sym}(3)$ *pure shear stress*, if T has the form

$$T^s = \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} = s(e_1 \otimes e_2 + e_2 \otimes e_1) \quad \text{with } s \in \mathbb{R} \text{ the amount of shear stress.} \quad (1.2)$$

In the following, we will discuss the incompatibility of simple shear deformations and pure shear stress in nonlinear Cauchy elasticity [18, 6, 15]. As a substitute, we introduce the apparently new concept of (idealized) *left* and *right finite simple shear deformation* shown in Figure 3 as well as the (idealized) *finite pure shear stretch*.¹

¹The class of finite pure shear stretches is a multiplicative group [beygelzimer2015equivalent], also known as the *group of hyperbolic rotations*. Note that $\exp \begin{pmatrix} 0 & \alpha \\ \alpha & 0 \end{pmatrix} * \exp \begin{pmatrix} 0 & \beta \\ \beta & 0 \end{pmatrix} = \exp \begin{pmatrix} 0 & \alpha + \beta \\ \alpha + \beta & 0 \end{pmatrix}$ since $\begin{pmatrix} 0 & \alpha \\ \alpha & 0 \end{pmatrix}, \begin{pmatrix} 0 & \beta \\ \beta & 0 \end{pmatrix}$ commute, cf. [28, p. 39] and Figure 4. To our knowledge, the finite pure shear stretch (1.5) was first mentioned by Claude Vallée [39] as the stretch induced by a Hencky-type logarithmic stress-strain relation under Cauchy pure shear stress.

Definitions 1.2. We call $F \in GL^+(3)$ an (idealized) *left finite simple shear deformation gradient* if F has the form

$$F_\alpha = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} 1 & \sinh(2\alpha) & 0 \\ 0 & \cosh(2\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R} \quad (1.3)$$

and an (idealized) *right finite simple shear deformation gradient* if F has the form

$$F_\alpha = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(2\alpha) & \sinh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R}. \quad (1.4)$$

Moreover, we call $V \in \text{Sym}^+(3)$ a *finite pure shear stretch* if V has the form

$$V_\alpha = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix} = \exp \begin{pmatrix} 0 & \alpha & 0 \\ \alpha & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R}. \quad (1.5)$$

Lemma 3.8 states that for given $\alpha \in \mathbb{R}$, the left and the right finite simple shear deformation are composed of exactly the same stretch and rotation, but applied in different order. Both the left and right finite simple shear deformations can be considered as finite extensions of the simple shear deformation due to their linearization (see Appendix B); the canonical identification of the amount of shear in the infinitesimal case is $\gamma = 2\alpha$.

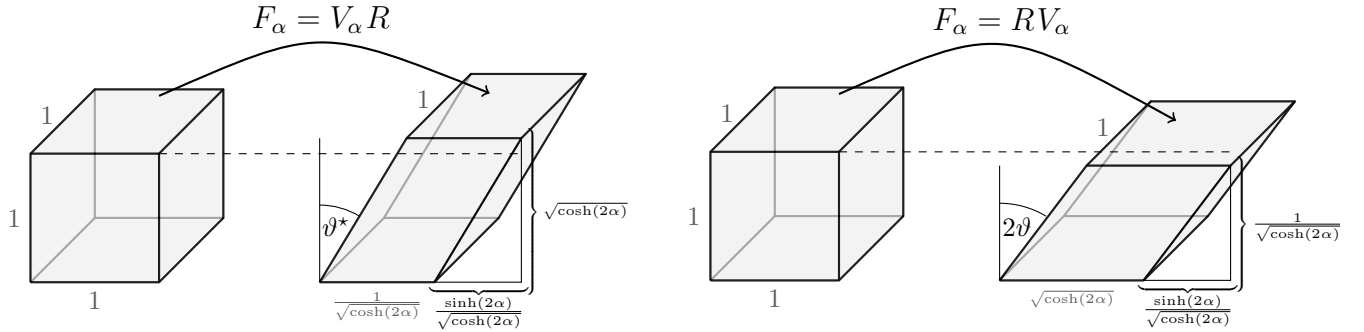


Figure 3: (left figure:) Left finite simple shear deformation and (right figure:) right finite simple shear deformation with $\tan(\vartheta^*) = \tanh(2\alpha)$ and $\tan(\vartheta) = \tanh(\alpha)$.

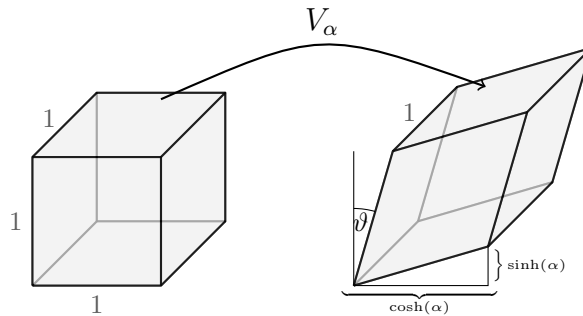


Figure 4: Finite pure shear stretch with $\tan(\vartheta) = \tanh(\alpha)$.

1.3 Shear in linear elasticity

In isotropic linear elasticity, the stress response is induced by the quadratic energy density function $W_{\text{lin}}(\varepsilon) = \mu \|\varepsilon\|^2 + \frac{\lambda}{2} \text{tr}(\varepsilon)^2 = \mu \|\text{dev } \varepsilon\|^2 + \frac{\kappa}{2} \text{tr}(\varepsilon)^2$, where $\varepsilon = \text{sym}(F - \mathbb{1})$, $\text{dev}(X) = X - \frac{1}{3} \text{tr}(X)\mathbb{1}$; here, μ denotes the infinitesimal shear modulus, $\kappa > 0$ is the infinitesimal bulk modulus, and λ is the first Lamé parameter. The corresponding linear Cauchy stress tensor is given by $\sigma_{\text{lin}} = 2\mu \text{dev } \varepsilon + \kappa \text{tr}(\varepsilon)\mathbb{1} = 2\mu\varepsilon + \lambda \text{tr}(\varepsilon)\mathbb{1}$. The following proposition describes the well-known relation between simple shear deformations and pure shear stress in the isotropic linear elasticity model.

Proposition 1.3. *The linear Cauchy shear stress σ_{lin} induced by a deformation $F \in \text{GL}^+(3)$ is a pure shear stress if and only if the deformation has the form*

$$F = \mathbb{1} + \underbrace{\begin{pmatrix} 0 & \frac{\gamma}{2} & 0 \\ \frac{\gamma}{2} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}}_{\varepsilon} + A \quad \text{with } \gamma \in \mathbb{R} \text{ and arbitrary } A \in \mathfrak{so}(3). \quad (1.6)$$

Proof. The relation $\sigma_{\text{lin}} = 2\mu \text{dev } \varepsilon + \kappa \text{tr}(\varepsilon)\mathbb{1}$ immediately implies that a deformation of the form (1.6) induces an infinitesimal Cauchy pure shear stress tensor with the amount of shear stress $s = \mu\gamma$.

On the other hand, if σ_{lin} is a pure shear stress, then $0 = \text{tr}(\sigma_{\text{lin}}) = \text{tr}(2\mu \text{dev } \varepsilon + \kappa \text{tr}(\varepsilon)\mathbb{1}) = 3\kappa \text{tr}(\varepsilon)$. Thus $\sigma_{\text{lin}} = 2\mu\varepsilon$, i.e.

$$2\mu\varepsilon = \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \implies \quad \varepsilon = \begin{pmatrix} 0 & \frac{s}{2\mu} & 0 \\ \frac{s}{2\mu} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix},$$

which leads to a deformation of the form (1.6) with $\gamma = \frac{s}{\mu}$ and arbitrary $A \in \mathfrak{so}(3)$. ■

Remark 1.4. Every simple shear deformation (1.1) has the form (1.6) and therefore leads to an infinitesimal pure shear stress tensor σ_{lin} :

$$F_\gamma = \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} = \mathbb{1} + \begin{pmatrix} 0 & \gamma & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} = \mathbb{1} + \underbrace{\begin{pmatrix} 0 & \frac{\gamma}{2} & 0 \\ \frac{\gamma}{2} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}}_{\varepsilon_\gamma \in \text{Sym}(3)} + \underbrace{\begin{pmatrix} 0 & \frac{\gamma}{2} & 0 \\ -\frac{\gamma}{2} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}}_{\omega_\gamma \in \mathfrak{so}(3)} = \mathbb{1} + \varepsilon_\gamma + \omega_\gamma \quad \text{with } \gamma \in \mathbb{R}.$$

Thus it is possible to **additively split** every deformation of the form (1.6), which leads to a pure shear stress in the isotropic linear model, as follows (cf. Figure 5):

Definitions 1.5. We call $\varepsilon \in \text{Sym}(3)$ an *infinitesimal pure shear strain* if ε has the form

$$\varepsilon_\gamma = \begin{pmatrix} 0 & \frac{\gamma}{2} & 0 \\ \frac{\gamma}{2} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \text{with } \gamma \in \mathbb{R} \text{ the amount of shear,} \quad (1.7)$$

and $\omega \in \mathfrak{so}(3)$ an *infinitesimal rotation of simple shear* if ω has the form

$$\omega_\gamma = \begin{pmatrix} 0 & \frac{\gamma}{2} & 0 \\ -\frac{\gamma}{2} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \text{with } \gamma \in \mathbb{R} \text{ the amount of shear.} \quad (1.8)$$

In particular, the finite pure shear stretch V_α (1.5) is the matrix exponential extension of the infinitesimal pure shear strain ε_γ , i.e. $V_\alpha = \exp(\varepsilon_\gamma)$ with $\gamma = 2\alpha$.

We observe that $F_\gamma = \mathbb{1} + \varepsilon_\gamma + \omega_\gamma$ is of the form (1.1) and that

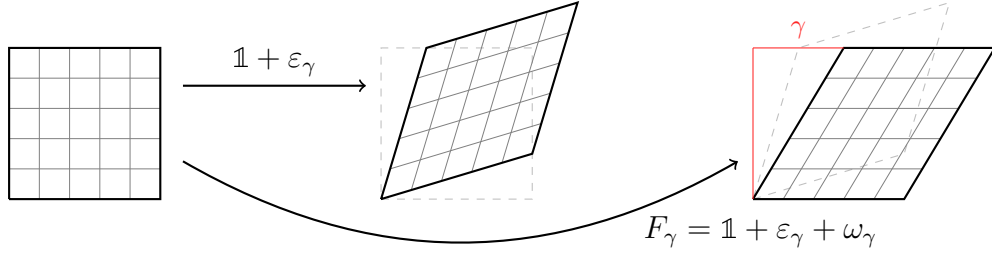


Figure 5: Additive split of the simple shear deformation F_γ into infinitesimal pure shear strain ε_γ and infinitesimal rotation ω_γ .

- i) the deformation F_γ is *infinitesimally volume preserving*, i.e. $\text{tr}(F_\gamma - \mathbb{1}) = 0$,
- ii) the deformation F_γ is *planar*, i.e. F_γ has the eigenvalue 1 to the eigenvector e_3 ,
- iii) the deformation F_γ is *ground parallel*, i.e. e_1 and e_3 are eigenvectors of F_γ .

Remark 1.6. Note carefully that the property $\det F_\gamma = 1$ is of no consequence in linear elasticity since, in contrast to the finite case, the infinitesimal change of volume induced by the deformation gradient F is described by $\text{tr}(\varepsilon) = \text{tr}(F) - 3$ and not the determinant of F . In particular, for $\gamma \neq 0$, the corresponding infinitesimal pure shear strain $\varepsilon_\gamma = \text{sym}(F_\gamma - \mathbb{1})$ is *not* finitely volume preserving, i.e. $\det(\mathbb{1} + \varepsilon_\gamma) \neq 1$.

The “accidental” fact that the simple shear deformation (1.1) is volume preserving in the finite sense as well appears to be a major source of confusion, since it suggests that in nonlinear elasticity, a “shear” deformation should have the exact form F_γ . However, as the result by Destrade et al. [6] clearly demonstrates, the finite generalization of simple shear deformations must take another form. In Section 3, we will show that deformations of the idealized form (1.3) are not only compatible with the linearized shear F_γ and generalize the properties i)–iii), but can be compatible with Cauchy pure shear stresses as well.

2 Shear in nonlinear elasticity

The following theorem summarizes the aforementioned result by Destrade et al. [6].

Theorem 2.1 (Destrade, Murphy and Saccomandi [6]). *Consider an isotropic elasticity law that satisfies the empirical inequalities [33, 36, 35, 18, 37]*

$$\beta_0 \leq 0, \quad \beta_1 > 0, \quad \beta_{-1} \leq 0 \quad \text{with} \quad \sigma = \beta_0 \mathbb{1} + \beta_1 B + \beta_{-1} B^{-1}, \quad (2.1)$$

also called the empirical true stress stretch (E-TSS)-relation [23]. If, for $F \in \text{GL}(3)$, $\hat{\sigma}(FF^T) = \sigma^s$ is a Cauchy pure shear stress, then there exists a triaxial stretch $\text{diag}(a, b, c)$ and a simple shear deformation F_γ such that $F = F_\gamma \cdot \text{diag}(a, b, c) \cdot Q$ for some $Q \in \text{SO}(3)$. Furthermore, if $s \neq 0$ for the amount of stress s , then $\text{diag}(a, b, c) \neq \mathbb{1}$, i.e. a simple shear deformation $F_\gamma \neq \mathbb{1}$ alone never corresponds to a nontrivial Cauchy pure shear stress of the form (1.2).

Destrade et al. implicitly utilize the so-called *semi-invertibility* [35] of the Cauchy stress response function $\hat{\sigma}: FF^T = B \mapsto \hat{\sigma}(B) = \sigma(\sqrt{FF^T})$, i.e. the representability

$$B = \psi_0(B)\mathbb{1} + \psi_1(B)\sigma + \psi_2(B)\sigma^2 \quad \text{for all } B \in \text{Sym}^+(3) \quad (2.2)$$

which is ensured² by the empirical inequalities (2.1). Note that (2.2) does not imply that the Cauchy stress-stretch law is invertible since the functions ψ_i still depend on B .

²It should be noted that the much weaker condition of the strict Baker-Ericksen inequalities (BE⁺) [1] would have been sufficient as well [7], cf. the implication chain “(E-TSS) \implies (WE-TSS) \implies (BE⁺) \implies (semi)” from [31, p. 135].

Remark 2.2. It is important to note that the condition of semi-invertibility, whether it is assumed directly or indirectly, always restricts the considered class of elasticity laws. In the following, we waive this restriction do not require any constitutive conditions to hold except the isotropy of the elasticity law and the uniqueness of the stress-free reference configuration, i.e. $\widehat{\sigma}(B) = 0$ if and only if $B = \mathbb{1}$.

Remark 2.3. A more general version of this result stated in Corollary 2.9 was given by Moon and Truesdell [18, (2.8)] and, implicitly, by Mihai and Goriely, cf. Remark 2.11.

Our computations are similar in part to those y Destrade et al.; however, instead of requiring the semi-invertibility (2.2), we utilize the commutativity of the left Cauchy-Green deformation tensor $B = FF^T$ with the Cauchy stress tensor $\widehat{\sigma}(B)$ which holds for **any** isotropic stress response. Thereby, in Lemma 2.5, we are able to determine the general form of all B which commute with a Cauchy pure shear stress which, in turn, allows us to compute the general form of the deformation gradient $F = F_\gamma \cdot \text{diag}(a, b, c) \cdot Q$ in Proposition 2.8. For a more detailed description regarding semi-invertibility and related matrix properties, see [32].

Proposition 2.4. *Let $T \in \text{Sym}^+(3)$ be a pure shear stress of the form (1.2). Then T has the eigenvalues $s, -s, 0$ with the corresponding eigenvectors $(\frac{\sqrt{2}}{2}, \frac{\sqrt{2}}{2}, 0)^T$, $(-\frac{\sqrt{2}}{2}, \frac{\sqrt{2}}{2}, 0)^T$ and $(0, 0, 1)^T$. In particular, T can be diagonalized to $Q \text{diag}(s, -s, 0)Q^T = T$ with*

$$Q := \frac{\sqrt{2}}{2} \begin{pmatrix} 1 & -1 & 0 \\ 1 & 1 & 0 \\ 0 & 0 & \sqrt{2} \end{pmatrix} \in \text{SO}(3). \quad (2.3)$$

Proof. We simply check

$$Q \begin{pmatrix} s & 0 & 0 \\ 0 & -s & 0 \\ 0 & 0 & 0 \end{pmatrix} Q^T = \frac{1}{2} \begin{pmatrix} 1 & -1 & 0 \\ 1 & 1 & 0 \\ 0 & 0 & \sqrt{2} \end{pmatrix} \begin{pmatrix} s & 0 & 0 \\ 0 & -s & 0 \\ 0 & 0 & 0 \end{pmatrix} \begin{pmatrix} 1 & 1 & 0 \\ -1 & 1 & 0 \\ 0 & 0 & \sqrt{2} \end{pmatrix} = \frac{1}{2} \begin{pmatrix} 0 & 2s & 0 \\ 2s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} = T. \quad \blacksquare$$

In particular, a pure shear stress tensor is either trivial (the case of the stress-free zero tensor) or has **three distinct eigenvalues**.

Let $\widehat{\sigma}(B)$ be a pure shear stress for arbitrary $B = FF^T$ with $F \in \text{GL}^+(3)$. Since the assumed isotropy ensures that B must commute with $\widehat{\sigma}(B)$, we consider the following lemma in order to deduce the specific form of B .

Lemma 2.5. *Let T be a pure shear stress tensor with $s \neq 0$ and $P \in \text{Sym}^+(3)$. Then P commutes with T if and only if P has the form*

$$P = \begin{pmatrix} p & q & 0 \\ q & p & 0 \\ 0 & 0 & r \end{pmatrix}. \quad (2.4)$$

Furthermore, $p = \frac{1}{2}(\mu_1 + \mu_2)$, $q = \frac{1}{2}(\mu_1 - \mu_2)$ and $r = \mu_3$, where $\mu_1, \mu_2, \mu_3 \in \mathbb{R}^+$ are the eigenvalues of P .

Proof. Suppose P and T commute. Then P and T are simultaneously diagonalizable [32, prop. 2.1]. Furthermore, if T has distinct eigenvalues, then T is coaxial with P , i.e. each eigenvector of T is an eigenvector of P . According to Proposition 2.4, T can be diagonalized using Q as in (2.3) and that the eigenvalues of T are distinct if and only if $s \neq 0$, which holds by assumption. Therefore, $Q^T P Q$ must be in diagonal form as well. With $\mu_1, \mu_2, \mu_3 \in \mathbb{R}$ denoting the eigenvalues of P we find

$$\begin{aligned} P &= Q \begin{pmatrix} \mu_1 & 0 & 0 \\ 0 & \mu_2 & 0 \\ 0 & 0 & \mu_3 \end{pmatrix} Q^T = \frac{1}{2} \begin{pmatrix} 1 & -1 & 0 \\ 1 & 1 & 0 \\ 0 & 0 & \sqrt{2} \end{pmatrix} \begin{pmatrix} \mu_1 & 0 & 0 \\ 0 & \mu_2 & 0 \\ 0 & 0 & \mu_3 \end{pmatrix} \begin{pmatrix} 1 & 1 & 0 \\ -1 & 1 & 0 \\ 0 & 0 & \sqrt{2} \end{pmatrix} \\ &= \frac{1}{2} \begin{pmatrix} 1 & -1 & 0 \\ 1 & 1 & 0 \\ 0 & 0 & \sqrt{2} \end{pmatrix} \begin{pmatrix} \mu_1 & \mu_1 & 0 \\ -\mu_2 & \mu_2 & 0 \\ 0 & 0 & \sqrt{2}\mu_3 \end{pmatrix} = \frac{1}{2} \begin{pmatrix} \mu_1 + \mu_2 & \mu_1 - \mu_2 & 0 \\ \mu_1 - \mu_2 & \mu_1 + \mu_2 & 0 \\ 0 & 0 & 2\mu_3 \end{pmatrix}, \end{aligned}$$

thus P is of the form (2.4).

Conversely, if P is of the form (2.4), then

$$PT = \begin{pmatrix} p & q & 0 \\ q & p & 0 \\ 0 & 0 & r \end{pmatrix} \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} = \begin{pmatrix} qs & ps & 0 \\ ps & qs & 0 \\ 0 & 0 & 0 \end{pmatrix} = \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \begin{pmatrix} p & q & 0 \\ q & p & 0 \\ 0 & 0 & r \end{pmatrix} = TP. \quad \blacksquare$$

Remark 2.6. Note that the proof of Lemma 2.5 makes use of the fact that a (non-trivial) pure shear stress has only simple eigenvalues; otherwise, it would not be possible to conclude that each eigenvector of T is an eigenvector of P without additional assumptions.

In terms of elasticity tensors, this observation can be stated as follows: although for an isotropic material, each principal axis of (Eulerian) strain must be a principal axis of (Cauchy) stress, the reverse must not hold in general unless the principal stresses are pairwise distinct or additional constraints on the constitutive law are assumed to hold. Among the constitutive requirements which guarantee this “bi-coaxiality” of stress and strain are the (weak) empirical inequalities [34, 35, 37], although the (weaker) strict Baker-Ericksen inequalities (BE^+) are sufficient as well [7]. Moreover, “bi-coaxiality” is equivalent to semi-invertibility (2.2).

According to Lemma 2.5, in order for $B = FF^T$ or $C = F^TF$ to commute with a pure shear stress tensor T , the Cauchy-Green deformation tensors B, C must be of the form

$$P = \begin{pmatrix} p & q & 0 \\ q & p & 0 \\ 0 & 0 & r \end{pmatrix} = \frac{1}{2} \begin{pmatrix} \lambda_1^2 + \lambda_2^2 & \lambda_1^2 - \lambda_2^2 & 0 \\ \lambda_1^2 - \lambda_2^2 & \lambda_1^2 + \lambda_2^2 & 0 \\ 0 & 0 & 2\lambda_3^2 \end{pmatrix} \quad (2.5)$$

with arbitrary singular values $\lambda_1, \lambda_2, \lambda_3$ of F , i.e. eigenvalues $\lambda_1^2, \lambda_2^2, \lambda_3^2$ of B and C . Moreover,

$$\sqrt{P} = \begin{pmatrix} p & q & 0 \\ q & p & 0 \\ 0 & 0 & r \end{pmatrix}^{1/2} = \frac{1}{2} \begin{pmatrix} \lambda_1 + \lambda_2 & \lambda_1 - \lambda_2 & 0 \\ \lambda_1 - \lambda_2 & \lambda_1 + \lambda_2 & 0 \\ 0 & 0 & 2\lambda_3 \end{pmatrix}. \quad (2.6)$$

Using this representation, we can now determine the general form of all $F \in GL^+(3)$ (independent of the particular elasticity law) which are able to correspond to a Cauchy-Green tensor of the form (2.5) and thus to a Cauchy pure shear stress. In particular, we can confirm that F cannot have the form of a simple shear deformation:

Remark 2.7. Let F be a simple shear deformation gradient with $\gamma \neq 0$ and T a pure shear stress tensor with $s \neq 0$. Because

$$FF^T = \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} 1 & 0 & 0 \\ \gamma & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} = \begin{pmatrix} 1 + \gamma^2 & \gamma & 0 \\ \gamma & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad (2.7)$$

and

$$F^TF = \begin{pmatrix} 1 & 0 & 0 \\ \gamma & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} = \begin{pmatrix} 1 & \gamma & 0 \\ \gamma & 1 + \gamma^2 & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad (2.8)$$

are both not of the form (2.4), neither $B = FF^T$ or $C = F^TF$ commutes with T .

Instead, up to an arbitrary pure rotation, F must take the form of a simple shear composed with a *triaxial stretch*. Now let $\hat{\sigma}(B)$ be a Cauchy pure shear stress. Then due to Lemma 2.5, the left Cauchy-Green deformation tensor B must be of the form (2.4), which allows us to compute the general form of F itself:

Proposition 2.8. Let $B \in \text{Sym}^+(3)$ be given by (2.4) with $p > |q|$ and $r > 0$. Then $F \in GL^+(3)$ with $FF^T = B$ is uniquely determined by

$$F = F_\gamma \text{diag}(a, b, c) Q = \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} Q, \quad (2.9)$$

up to an arbitrary $Q \in \text{SO}(3)$, where

$$a = \sqrt{\frac{p^2 - q^2}{p}}, \quad b = \sqrt{p} = \sqrt{B_{11}}, \quad c = \sqrt{r} \quad \text{and} \quad \gamma = \frac{q}{p} \quad (2.10)$$

or, in terms of the singular values $\lambda_1, \lambda_2, \lambda_3 \in \mathbb{R}_+$ of F ,

$$a = \lambda_1 \lambda_2 \sqrt{\frac{2}{\lambda_1^2 + \lambda_2^2}}, \quad b = \sqrt{\frac{\lambda_1^2 + \lambda_2^2}{2}}, \quad c = \lambda_3, \quad \gamma = \frac{\lambda_1^2 - \lambda_2^2}{\lambda_1^2 + \lambda_2^2}. \quad (2.11)$$

In particular, F is necessarily of the form (2.9) if the Cauchy stress tensor corresponding to the deformation gradient F induced by an isotropic law of Cauchy elasticity is a pure shear stress tensor of the form $\sigma = s(e_1 \otimes e_2 + e_2 \otimes e_1)$ with $s \in \mathbb{R}$.

Proof. Let $\tilde{F} = F_\gamma \text{diag}(a, b, c)$ with a, b, c given by (2.10). Then $a^2 + b^2\gamma^2 = b^2 = p$, $b^2\gamma = q$ and $c^2 = r$, thus

$$\tilde{F}\tilde{F}^T = \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} \begin{pmatrix} 1 & 0 & 0 \\ \gamma & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} = \begin{pmatrix} a^2 + b^2\gamma^2 & b^2\gamma & 0 \\ b^2\gamma & b^2 & 0 \\ 0 & 0 & c^2 \end{pmatrix} = \begin{pmatrix} p & q & 0 \\ q & p & 0 \\ 0 & 0 & r \end{pmatrix}.$$

Due to Lemma 2.5, $p = \frac{1}{2}(\lambda_1^2 + \lambda_2^2)$, $q = \frac{1}{2}(\lambda_1^2 - \lambda_2^2)$ and $r = \lambda_3^2$, which immediately implies $b = \sqrt{\frac{\lambda_1^2 + \lambda_2^2}{2}}$, $c = \lambda_3$ and $\gamma = \frac{\lambda_1^2 - \lambda_2^2}{\lambda_1^2 + \lambda_2^2}$. Moreover, $p + q = \lambda_1^2$ and $p - q = \lambda_2^2$, thus $p^2 - q^2 = \lambda_1^2 \lambda_2^2$ and therefore $a = \lambda_1 \lambda_2 \sqrt{\frac{2}{\lambda_1^2 + \lambda_2^2}}$.

Now let B be given by (2.4) and consider an arbitrary $F \in \text{GL}^+(3)$ with $FF^T = B = \tilde{F}\tilde{F}^T$. Since F is uniquely determined by FF^T up to a right-hand rotation, there exists $Q \in \text{SO}(3)$ with $F = \tilde{F}Q$, thus F is of the form (2.9). \blacksquare

The following generalization of Theorem 2.1 summarizes the above considerations.

Corollary 2.9. *Consider an isotropic elasticity law with a unique stress-free reference configuration, i.e. $\hat{\sigma}(B) = 0$ if and only if $B = \mathbf{1}$. Then any deformation gradient F which induces a Cauchy pure shear stress $\sigma = s(e_1 \otimes e_2 + e_2 \otimes e_1)$ with $s \in \mathbb{R}$ is necessarily of the form (2.9).*

Proof. Due to the assumed isotropy, the stretch $B = FF^T$ and the stress $\hat{\sigma}(B)$ commute. According to Lemma 2.5, B must therefore be of the form (2.4), thus F must have the form (2.9) due to Proposition 2.8. \blacksquare

Remark 2.10. In order for the term $\sqrt{\frac{p^2 - q^2}{p}}$ to be well defined, the condition $p > |q|$ must hold. This implies the upper bound $|\gamma| = \frac{|q|}{p} < 1$, i.e. the shear angle is always limited by 45° . Note carefully that this limitation $p = \frac{1}{2}(\mu_1 + \mu_2) > \frac{1}{2}|\mu_1 - \mu_2| = q$ is due to the positive definiteness of B and not dependent on any constitutive requirements.

Remark 2.11. While (unlike Moon and Truesdell [18]) Mihai and Goriely [15] do not state the result of Corollary 2.9 in full generality, their proof does not require the empirical inequalities, the Baker-Ericksen inequalities or the semi-invertibility of the Cauchy stress response. Both the proofs given in [18] and [15] implicitly utilize the bi-coaxiality of the Cauchy stress tensor σ and the stretch tensor B and thus the distinctness of the eigenvalues, although they involve a more direct computation of the matrix entries of B instead of the diagonalization approach presented here.

The factor $b = \sqrt{B_{11}}$ in Proposition 2.8 plays an important role for the so-called *Poynting effect* [15, 16], which describes the observed change of axial length of a cylinder subjected to torsion [29]. The close connection of this effect to the material behaviour under shear stresses arises from considering infinitesimal cubes on the surface, visualized in Figure 1. A positive Poynting effect, i.e. an increase in axial length corresponding to a value $b > 1$, is

is exhibited by most physical materials, although a contraction in axial direction (negative Poynting effect, $b < 1$) has been observed for certain biopolymers [15].

Another effect connected to shear stresses is the so-called *Kelvin effect* [36, p. 176] which describes a change in the volume, i.e. $\det F \neq 1$, of the material under pure shear stress. Note that this behaviour cannot be described by a hyperelastic material with an additive isochoric-volumetric split, i.e. an energy potential of the form $W(F) = W_{\text{iso}}(F/(\det F)^{\frac{1}{3}}) + f(\det F)$, since in this case [30]

$$0 = \text{tr}(\sigma) = f'(\det V) \quad \implies \quad \det V = 1 \quad (2.12)$$

due to the usual requirement of a unique stress-free reference configuration.

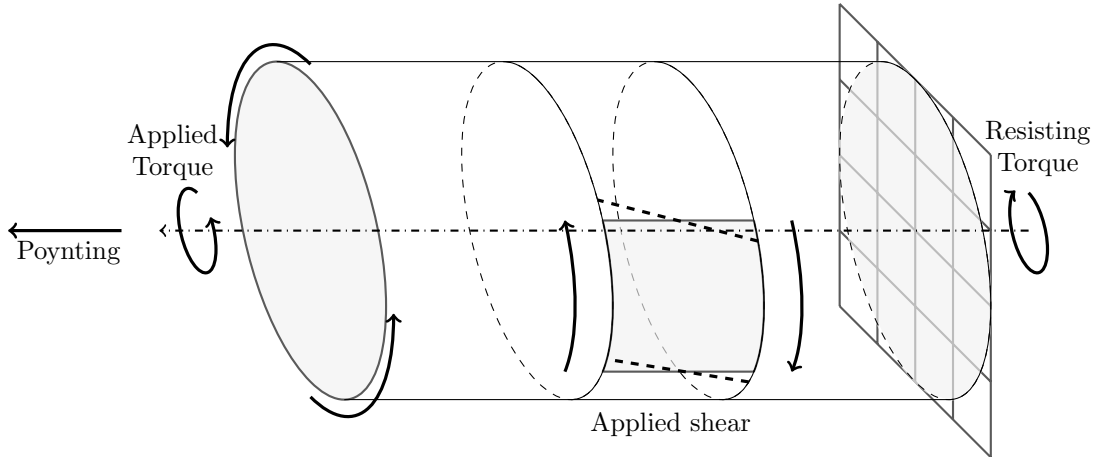


Figure 6: Infinitesimal planes tangential to the cylinder are subject to shear, and the generalized *positive* Poynting effect for these planes leads to an elongation in the direction of the torsional axis (positive Poynting effect).

We summarize:

Cauchy pure shear stress $\hat{\sigma}(B) = \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \longrightarrow B = \begin{pmatrix} B_{11} & B_{12} & 0 \\ B_{21} & B_{22} & 0 \\ 0 & 0 & B_{33} \end{pmatrix}$	
Positive Poynting effect: $B_{11} > 1$, Kelvin effect: $\det B \neq 1$,	Negative Poynting effect: $B_{11} < 1$. “not-planar”: $B_{33} \neq 1$.

Example 2.12. Consider the elastic energy potential W and the corresponding Cauchy stress response $\hat{\sigma}$ with

$$W(F) = \frac{1}{2} \|F\|^2 - \log(\det F) = \frac{1}{2} (I_1 - \log I_3), \quad \hat{\sigma}(B) = \frac{1}{\sqrt{\det B}} [B - \mathbb{1}]. \quad (2.13)$$

If $\sigma = s(e_1 \otimes e_2 + e_2 \otimes e_1)$ is a Cauchy pure shear stress, then the left Cauchy-Green deformation tensor B is of the form (2.4) and thus

$$\begin{aligned} \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} &= \frac{1}{\sqrt{I_3}} \begin{pmatrix} p-1 & q & 0 \\ q & p-1 & 0 \\ 0 & 0 & r-1 \end{pmatrix} \iff r=1, \quad p=1 \quad \text{and} \quad s = \frac{q}{\sqrt{I_3}} \\ \implies q &= s \sqrt{(p^2 - q^2)r} \stackrel{r=p=1}{\implies} q^2 = s^2(1 - q^2) \iff (1 + s^2) q^2 = s^2 \implies q = \frac{s}{\sqrt{1 + s^2}}. \end{aligned}$$

Thus the deformation is planar ($r = 1$) but not volume preserving for $s \neq 0$, since

$$\det B = (p^2 - q^2)r = 1 - \frac{s^2}{1 + s^2}. \quad \blacksquare$$

3 Idealized finite simple shear deformations

Of course, while any deformation gradient $F \in \text{GL}^+(3)$ corresponding to a Cauchy pure shear stress must be of the form (2.8) regardless of the (isotropic) constitutive law of elasticity, the value of the parameters a, b, c, γ or, equivalently, the principal stretches $\lambda_1, \lambda_2, \lambda_3$, depend on the specific stress-strain relation.

In particular, we believe that not every deformation of the general form (2.9) is suitable to be called “shear”, motivating the following definition.

Definition 3.1. We call $F = VR \in \text{GL}^+(3)$ with $V \in \text{Sym}^+(3)$ and $R \in \text{SO}(3)$ an (idealized) (finite) shear deformation if the following requirements are satisfied:

- i) The stretch V (or, equivalently, the deformation F) is *volume preserving*, i.e. $\det V = 1$.
- ii) The stretch V is *planar*, i.e. V has the eigenvalue 1 to the eigenvector e_3 .
- iii) The rotation R is such that the deformation F is *ground parallel*, i.e. e_1, e_3 are eigenvectors of F .

In terms of the singular values $\lambda_1, \lambda_2, \lambda_3 \in \mathbb{R}_+$ of F , i.e. the eigenvalues of V , the first two conditions can be stated as $\lambda_1 \lambda_2 \lambda_3 = 1$ and $\lambda_3 = 1$, respectively. In particular, i) and ii) are satisfied if and only if there exists $\lambda \in \mathbb{R}_+$ with $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$ and $\lambda_3 = 1$.

Remark 3.2. Definition 3.1 is a direct generalization of the infinitesimal behavior: the infinitesimal (classical) simple shear is planar, ground parallel and infinitesimally volume preserving, cf. Definition 1.5. The concept is visualized in Figure 9.

We will demonstrate that the deformations of the general triaxial form (2.9) which additionally satisfy the requirements of Definition 3.1 are exactly the left finite simple shear deformations introduced in Definition 1.2. Similarly, as shown in Appendix A, any shear deformation that corresponds to a Biot pure shear stress is a right finite simple shear deformation.

Lemma 3.3. Let T be a pure shear stress tensor with $s \neq 0$ and $P \in \text{Sym}^+(3)$ such that the eigenvalues $\lambda_1^2, \lambda_2^2, \lambda_3^2$ of P are given by $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$ and $\lambda_3 = 1$ for arbitrary $\lambda \in \mathbb{R}_+$. Then P and T commute if and only if P is of the form

$$\sqrt{P} = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix} = \exp \begin{pmatrix} 0 & \alpha & 0 \\ \alpha & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R} \quad (3.1)$$

i.e. if and only if \sqrt{P} is a finite pure shear stretch V_α of the form (1.5).

Proof. By Lemma 2.5, the tensor P commutes with T if and only if it is of the form (2.5). Let $\alpha := \log(\lambda) \in \mathbb{R}$. Then, for $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$ and $\lambda_3 = 1$, eq. (2.6) yields

$$\sqrt{P} = \frac{1}{2} \begin{pmatrix} \lambda_1 + \lambda_2 & \lambda_1 - \lambda_2 & 0 \\ \lambda_1 - \lambda_2 & \lambda_1 + \lambda_2 & 0 \\ 0 & 0 & 2\lambda_3 \end{pmatrix} = \frac{1}{2} \begin{pmatrix} e^\alpha + e^{-\alpha} & e^\alpha - e^{-\alpha} & 0 \\ e^\alpha - e^{-\alpha} & e^\alpha + e^{-\alpha} & 0 \\ 0 & 0 & 2 \end{pmatrix} = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix}. \quad \blacksquare$$

Remark 3.4. In contrast to infinitesimal pure shear strain (see Definition 1.5 and Remark 1.6), the finite pure shear stretch is finitely volume preserving as well (since $\det V_\alpha = 1$). This fact is visualized for selected γ and α in Figure 7: The determinant of infinitesimal pure shear stretch is $\det(\mathbb{1} + \varepsilon_\gamma) = 1 - \frac{\gamma^2}{4}$, whereas the determinant of finite pure shear stretch is 1. Note that finite pure shear stretch linearizes to linear pure shear stretch, cf. Appendix B.

Lemma 3.5. Let $B = P$ be given by (3.1). Then any deformation gradient $F \in \text{GL}^+(3)$ with $FF^T = B$ is of the

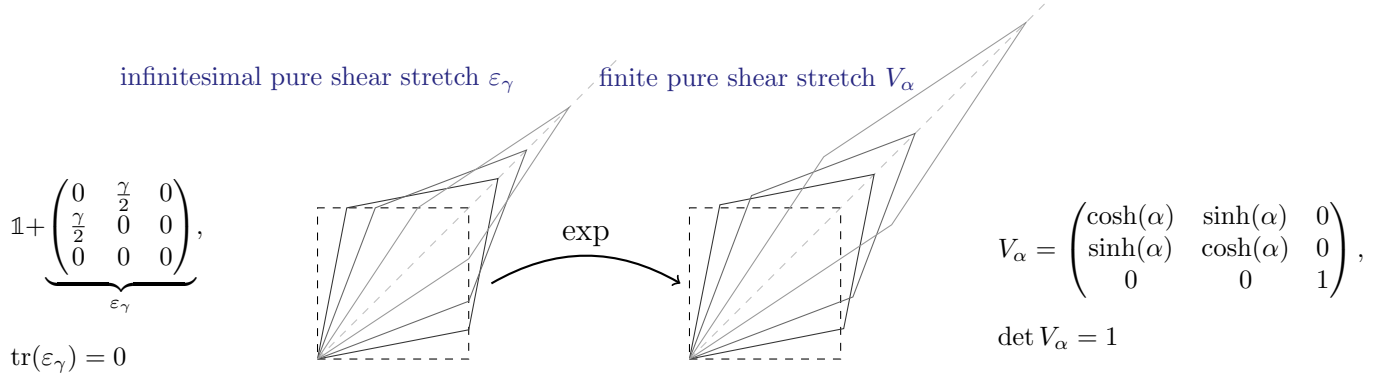


Figure 7: Linear and finite pure shear stretch with $\gamma = 2\alpha$. Both are infinitesimally volume preserving, but only the finite pure shear stretch leaves the volume unchanged for finite stretch ratios. The transition mechanism is the matrix exponential.

form

$$F = \begin{pmatrix} \frac{1}{\sqrt{\cosh(2\alpha)}} & \frac{\sinh(2\alpha)}{\sqrt{\cosh(2\alpha)}} & 0 \\ 0 & \sqrt{\cosh(2\alpha)} & 0 \\ 0 & 0 & 1 \end{pmatrix} \cdot Q = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} 1 & \sinh(2\alpha) & 0 \\ 0 & \cosh(2\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} \cdot Q \quad (3.2)$$

with arbitrary $Q \in \text{SO}(3)$.

Proof. We use Proposition 2.8 and write $\lambda_1 = e^\alpha$, $\lambda_2 = e^{-\alpha}$, $\lambda_3 = 1$. Then

$$\gamma = \frac{\lambda_1^2 - \lambda_2^2}{\lambda_1^2 + \lambda_2^2} = \frac{e^{2\alpha} - e^{-2\alpha}}{e^{2\alpha} + e^{-2\alpha}} = \tanh(2\alpha), \quad b = \sqrt{\frac{\lambda_1^2 + \lambda_2^2}{2}} = \sqrt{\frac{e^{2\alpha} + e^{-2\alpha}}{2}} = \sqrt{\cosh(2\alpha)},$$

$$a = \lambda_1 \lambda_2 \sqrt{\frac{2}{\lambda_1^2 + \lambda_2^2}} = e^\alpha e^{-\alpha} \cdot \sqrt{\frac{2}{e^{2\alpha} + e^{-2\alpha}}} = \frac{1}{\sqrt{\cosh(2\alpha)}} \quad \text{and} \quad c = \lambda_3 = 1$$

and thus

$$F = \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} Q = \begin{pmatrix} 1 & \tanh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} \frac{1}{\sqrt{\cosh(2\alpha)}} & 0 & 0 \\ 0 & \sqrt{\cosh(2\alpha)} & 0 \\ 0 & 0 & 1 \end{pmatrix} Q$$

$$= \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} 1 & \sinh(2\alpha) & 0 \\ 0 & \cosh(2\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} Q. \quad \blacksquare$$

In particular, the triaxial stretch $\text{diag}(a, b, c)$ has to be a (diagonal) biaxial pure shear stretch of the form $\text{diag}(a, \frac{1}{a}, 1)$ if the deformation is a finite shear in the sense of Definition 1.5. The exact form the deformation can take in this case is given by the following theorem.

Theorem 3.6. *Any idealized shear deformation³ $F \in \text{GL}^+(3)$ satisfying Definition 3.1 that corresponds to a (non-trivial) Cauchy pure shear stress for an isotropic law of elasticity is a left finite simple shear deformation of the form*

$$F_\alpha = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} 1 & \sinh(2\alpha) & 0 \\ 0 & \cosh(2\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R}. \quad (3.3)$$

³Again, that is: $\det F = 1$, e_3 is an eigenvector of F for eigenvalue 1 and e_1 is an eigenvector of F .

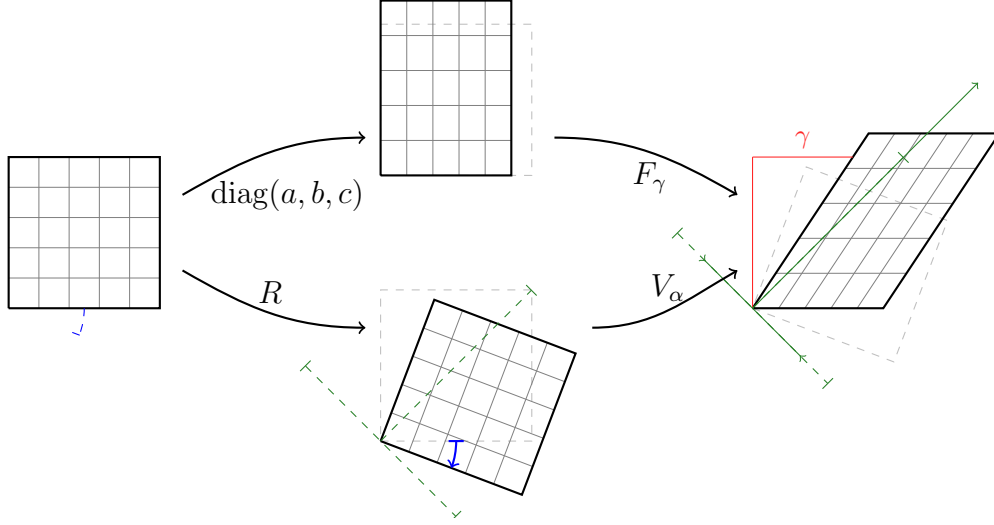


Figure 8: Left finite simple shear deformation with amount of shear $\gamma = \tanh(2\alpha)$, cf. [13].

Proof. Let $\hat{\sigma}$ be a pure shear stress tensor with $s \neq 0$ and $B \in \text{Sym}^+(3)$ with $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$, $\lambda_3 = 1$ for arbitrary $\lambda \in \mathbb{R}_+$ with $\lambda_1^2, \lambda_2^2, \lambda_3^2$ as the eigenvalues of B . By Lemma 3.3, the tensors B and $\hat{\sigma}(B)$ commute if and only if $B = V^2$ is of the form (3.1), i.e. if and only if

$$V = V_\alpha = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix} = \exp \begin{pmatrix} 0 & \alpha & 0 \\ \alpha & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R} \quad (3.4)$$

is a finite pure shear stretch.

For an isotropic elasticity law the left Cauchy-Green deformation tensor $B = FF^T$ commutes with $\hat{\sigma}(B)$. Thus if $\hat{\sigma}(B)$ is a Cauchy pure shear stress, then $B = FF^T$ is of the form (3.1) (i.e. $V = V_\alpha$ is a finite pure shear stress). By Lemma 3.5, the deformation gradient F is determined up to an arbitrary right-hand side rotation $Q \in \text{SO}(3)$, and it remains to observe that exactly for $Q = \mathbb{1}$, the resulting deformation F_α satisfies the condition of ground parallelism (i.e. has eigenvectors e_1 and e_3). ■

Observe that $b = \sqrt{\cosh(2\alpha)} = \sqrt{B_{11}} \geq 1$, hence a finite simple shear deformation F_α always results in a positive Poynting effect, which is considered a physically plausible behavior for most materials.

In Appendix A we prove the analogous result for Biot pure shear stress and right finite shear deformations:

Theorem 3.7 (Theorem 3.6). *Any shear deformation³ $F \in \text{GL}^+(3)$ that results in a (non-trivial) pure shear Biot stress tensor T^{Biot} is a right finite simple shear deformation*

$$F_\alpha = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(2\alpha) & \sinh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R}. \quad (3.5)$$

The following connection between the left and right finite simple shear deformation is visualized in Figure 8.

Lemma 3.8. *Let $\alpha \in \mathbb{R}$ and*

$$V_\alpha = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix}, \quad R = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ -\sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix}. \quad (3.6)$$

Then

$$V_\alpha R = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} 1 & \sinh(2\alpha) & 0 \\ 0 & \cosh(2\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix}, \quad RV_\alpha = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(2\alpha) & \sinh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} \quad (3.7)$$

are a left finite simple shear deformation and a right finite simple shear deformation, respectively.

Proof. The proof follows from direct computation, using the general equalities

$$\cosh(\alpha)^2 + \sinh(\alpha)^2 = \cosh(2\alpha) \quad \text{and} \quad 2 \sinh(\alpha) \cosh(\alpha) = \sinh(2\alpha). \quad \blacksquare$$

Remark 3.9. Lemma 3.8 states that for a given $\alpha \in \mathbb{R}$, the left and the right finite simple shear deformation are generated by exactly the same stretch V_α and rotation R , the only difference being the order of application of the two linear operations.

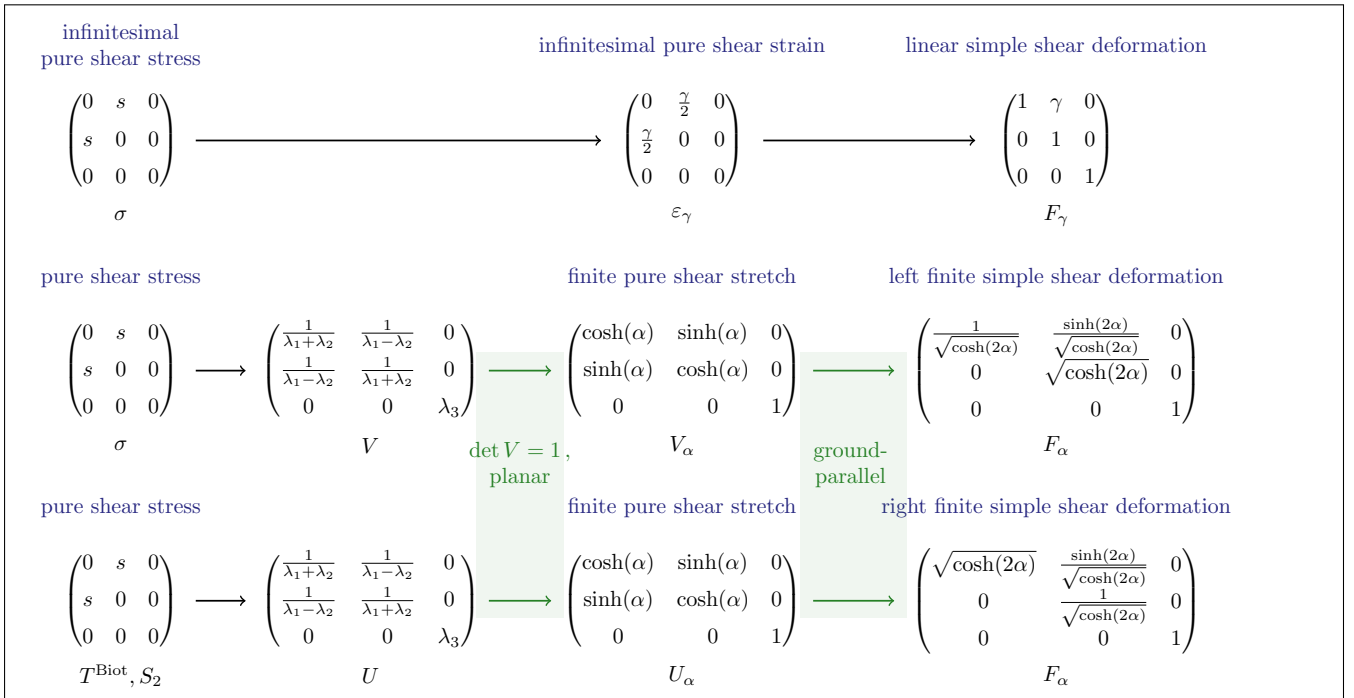


Figure 9: Comparison of pure shear stress and finite simple shear.

4 Constitutive conditions for idealized shear response in Cauchy pure shear stress

As stated in Corollary 2.9, a Cauchy pure shear stress corresponds with a deformation gradient F of the general triaxial form (2.9). Theorem 3.6 shows that if F_α also satisfies Definition 3.1, then F must be of the form (3.3). However, it is important to note that whether or not a deformation gradient F corresponding to a pure shear stress satisfies Definition 3.1 depends on the particular stress response function. Similarly, not every constitutive law ensures that every idealized finite shear of the form (3.3) induces a pure shear stress tensor.

On the other hand, a number of reasons suggest that for an *idealized* elastic material, finite simple shear deformations *should* correspond to pure shear Cauchy stresses; for example:

- Cauchy pure shear stress does not induce a pressure load, i.e. $\text{tr}(\sigma) = 0$. Therefore, it should be assumed that the pure stretch response is *volume-preserving* (no Kelvin effect).⁴ This corresponds to the case of infinitesimal shear, where the linear strain tensor ε is trace free.
- Cauchy pure shear stress acts only in the e_1 - e_2 -plane. Therefore, it is plausible (for an *idealized* shear) to assume a pure stretch response only in the corresponding plane and no deformation in the third component ($\lambda_3 = 1$). Again, this behavior is always exhibited in the linearized case.

In the following, we therefore examine constitutive conditions for the elasticity law which ensure that *every* left finite simple shear deformation F_α is mapped to a Cauchy pure shear stress. In Section 4.2, we will also discuss additional requirements which ensure the converse implication, i.e. conditions under which every Cauchy stress always induces a pure shear stretch. Again, note carefully that those two implications are *distinct* properties of an elastic response function (cf. [14]), since the mapping $V \mapsto \sigma(V) = \hat{\sigma}(V^2)$ is generally not invertible.

4.1 Pure shear stress induced by pure shear stretch

Recall from Lemma 2.5 that for any left finite simple shear deformation, all eigenvectors of $B = F_\alpha F_\alpha^T$ or V_α are eigenvectors of a Cauchy pure shear stress $\hat{\sigma}(FF^T)$. More specifically,

$$V_\alpha = \sqrt{F_\alpha F_\alpha^T} = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix} = Q \cdot \underbrace{\begin{pmatrix} \lambda & 0 & 0 \\ 0 & \frac{1}{\lambda} & 0 \\ 0 & 0 & 1 \end{pmatrix}}_{\text{pure shear deformation}} \cdot Q^T \quad \text{with} \quad \lambda = e^\alpha$$

and

$$\hat{\sigma}(B) = \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} = Q \begin{pmatrix} s & 0 & 0 \\ 0 & -s & 0 \\ 0 & 0 & 0 \end{pmatrix} Q^T \quad \text{with} \quad Q = \frac{\sqrt{2}}{2} \begin{pmatrix} 1 & -1 & 0 \\ 1 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \in \text{SO}(3).$$

Therefore, whether the Cauchy stress $\hat{\sigma}(FF^T)$ corresponding to a left finite simple shear (or, equivalently, the Cauchy stress $\hat{\sigma}(V^2)$ corresponding to a finite pure shear stretch $V = V_\alpha$ of the form (1.5)) is a pure shear stress depends only on the principal stresses $\sigma_1, \sigma_2, \sigma_3$. In particular, for a given stress response, a finite pure shear stretch *always* induces a pure shear stress *if and only if* for all $\lambda \in \mathbb{R}^+$, there exists $s \in \mathbb{R}$ such that

$$B = V^2 = \begin{pmatrix} \lambda^2 & 0 & 0 \\ 0 & \frac{1}{\lambda^2} & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad \implies \quad \hat{\sigma}(B) = \begin{pmatrix} s & 0 & 0 \\ 0 & -s & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad (4.1)$$

or, equivalently,

$$\lambda_1 = \lambda, \quad \lambda_2 = \frac{1}{\lambda}, \quad \lambda_3 = 1 \quad \implies \quad \sigma_1 = s, \quad \sigma_2 = -s, \quad \sigma_3 = 0,$$

where σ_i denotes the i -th eigenvalue of $\hat{\sigma}(FF^T)$ for $F = \text{diag}(\lambda_1, \lambda_2, \lambda_3)$.

Recall that any isotropic Cauchy stress response $B \mapsto \hat{\sigma}(B)$ can be expressed in the form [30]

$$\sigma = \beta_0 \mathbb{1} + \beta_1 B + \beta_{-1} B^{-1} \quad (4.2)$$

with scalar-valued functions $\beta_0, \beta_1, \beta_{-1}$ depending on the matrix invariants I_1, I_2, I_3 of the stretch tensor B . In terms of this representation, equality (4.1) reads

$$\begin{pmatrix} s & 0 & 0 \\ 0 & -s & 0 \\ 0 & 0 & 0 \end{pmatrix} = \beta_0 \begin{pmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} + \beta_1 \begin{pmatrix} \lambda^2 & 0 & 0 \\ 0 & \frac{1}{\lambda^2} & 0 \\ 0 & 0 & 1 \end{pmatrix} + \beta_{-1} \begin{pmatrix} \frac{1}{\lambda^2} & 0 & 0 \\ 0 & \lambda^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}. \quad (4.3)$$

⁴Moon and Truesdell [18, p. 2] remark that “[i]t is easily possible to conceive a *truly isotropic solid* that [...] retains its volume unchanged when subjected to a shear stress of any amount”; in the following, we give examples of such material models.

Assume without loss of generality that $\lambda \neq 1$ and let $s = \beta_0 + \beta_1 \lambda^2 + \beta_{-1} \frac{1}{\lambda^2}$. Then (4.3) is equivalent to the equations

$$-\left(\beta_0 + \beta_1 \lambda^2 + \beta_{-1} \frac{1}{\lambda^2}\right) = \beta_0 + \beta_1 \frac{1}{\lambda^2} + \beta_{-1} \lambda^2 \quad \text{and} \quad 0 = \beta_0 + \beta_1 + \beta_{-1}. \quad (4.4)$$

Using the second equation, we can simplify the first one to

$$0 = 2\beta_0 + \beta_1 \left(\lambda^2 + \frac{1}{\lambda^2}\right) + \beta_{-1} \left(\frac{1}{\lambda^2} + \lambda^2\right) = 2 \underbrace{(\beta_0 + \beta_1 + \beta_{-1})}_{=0} + \underbrace{(\lambda^2 + \lambda^{-2} - 2)}_{>0} (\beta_1 + \beta_{-1}),$$

i.e. to $\beta_1 + \beta_{-1} = 0$, which reduces the second equation (4.4)₂ to $\beta_0 = 0$. The condition (4.4) can therefore be stated as

$$\beta_1 + \beta_{-1} = 0 \quad \text{and} \quad \beta_0 = 0 \quad \text{for all } \lambda \in \mathbb{R}^+ \text{ with } \lambda_1 = \lambda, \lambda_2 = \frac{1}{\lambda}, \lambda_3 = 1. \quad (4.5)$$

In the following, we will assume that the elasticity law is *hyperelastic*, i.e. induced by an elastic energy function $W: \text{GL}^+(3) \rightarrow \mathbb{R}$. Then the functions β_i can be expressed by

$$\beta_0 = \frac{2}{\sqrt{I_3}} \left(I_2 \frac{\partial W}{\partial I_2} + I_3 \frac{\partial W}{\partial I_3} \right), \quad \beta_1 = \frac{2}{\sqrt{I_3}} \frac{\partial W}{\partial I_1}, \quad \beta_{-1} = -2\sqrt{I_3} \frac{\partial W}{\partial I_2} \quad (4.6)$$

with the matrix invariants

$$I_1 = \text{tr } B = \lambda_1^2 + \lambda_2^2 + \lambda_3^2, \quad I_2 = \text{tr}(\text{Cof } B) = \lambda_1^2 \lambda_2^2 + \lambda_1^2 \lambda_3^2 + \lambda_2^2 \lambda_3^2, \quad I_3 = \det B = \lambda_1^2 \lambda_2^2 \lambda_3^2. \quad (4.7)$$

of B . If the singular values of F are given by $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$ and $\lambda_3 = 1$, then $I_1 = I_2 = 1 + \lambda^2 + \frac{1}{\lambda^2} \geq 3$ and $I_3 = 1$. The compatibility conditions (4.5) can be restated in terms of the hyperelastic energy function:

$$\begin{aligned} & \beta_1 + \beta_{-1} = 0 \quad \text{and} \quad \beta_0 = 0 \quad \text{for all } I_1 = I_2 \geq 3, I_3 = 1 \\ \iff & \frac{2}{\sqrt{I_3}} \frac{\partial W}{\partial I_1} = 2\sqrt{I_3} \frac{\partial W}{\partial I_2} \quad \text{and} \quad \frac{2}{\sqrt{I_3}} \left(I_2 \frac{\partial W}{\partial I_2} + I_3 \frac{\partial W}{\partial I_3} \right) = 0 \quad \text{for all } I_1 = I_2 \geq 3, I_3 = 1 \end{aligned}$$

or, equivalently,

$$\frac{\partial W}{\partial I_1} = \frac{\partial W}{\partial I_2} \quad \text{and} \quad I_2 \frac{\partial W}{\partial I_2} + \frac{\partial W}{\partial I_3} = 0 \quad \text{for all } I_1 = I_2 \geq 3, I_3 = 1. \quad (4.8)$$

Many isotropic energy functions are expressed more conveniently in terms of the principal stretches instead of the three matrix invariants I_1 , I_2 and I_3 , i.e. in the form

$$W(F) = W(\lambda_1, \lambda_2, \lambda_3) \quad \text{for all } F \in \text{GL}^+(3) \text{ with singular values } \lambda_1, \lambda_2, \lambda_3. \quad (4.9)$$

The isotropy implies that this representation is invariant under permutations of the arguments. In order to find conditions on W which ensure that our finite pure shear stretches correspond to pure shear stresses, we use the general formula [27, p.216]

$$\sigma_i = \frac{\lambda_i}{\lambda_1 \lambda_2 \lambda_3} \frac{\partial W}{\partial \lambda_i}(\lambda_1, \lambda_2, \lambda_3). \quad (4.10)$$

for the eigenvalues of $\hat{\sigma}(B)$. Again, we want to ensure that (4.1) holds. Similar to (4.5), this leads to the two equations

$$\sigma_1 + \sigma_2 = 0 \quad \text{and} \quad \sigma_3 = 0 \quad \text{for all } \lambda \in \mathbb{R}^+ \text{ with } \lambda_1 = \lambda, \lambda_2 = \frac{1}{\lambda}, \lambda_3 = 1. \quad (4.11)$$

With (4.10), these two compatibility conditions read

$$\lambda \frac{\partial W}{\partial \lambda_1} + \frac{1}{\lambda} \frac{\partial W}{\partial \lambda_2} = 0 \quad \text{and} \quad \frac{\partial W}{\partial \lambda_3} = 0 \quad \text{for all } \lambda \in \mathbb{R}^+ \text{ with } \lambda_1 = \lambda, \lambda_2 = \frac{1}{\lambda}, \lambda_3 = 1. \quad (4.12)$$

The following Lemma gives a simple sufficient criterion for eq. (4.12) and thus for the desired implication (4.1).

Lemma 4.1. *Let $W: \text{GL}^+(3) \rightarrow \mathbb{R}$ be a sufficiently smooth isotropic elastic energy potential. If W is tension-compression symmetric, i.e. if $W(F^{-1}) = W(F)$ for all $F \in \text{GL}^+(3)$, then $\hat{\sigma}(B) = \hat{\sigma}(V^2)$ is a pure shear stress for every finite pure shear stretch $V = V_\alpha$.*

Proof. In terms of the singular values, the condition of tension-compression symmetry reads

$$W\left(\frac{1}{\lambda_1}, \frac{1}{\lambda_2}, \frac{1}{\lambda_3}\right) = W(\lambda_1, \lambda_2, \lambda_3) \quad \text{for all } \lambda_1, \lambda_2, \lambda_3 \in \mathbb{R}^+, \quad (4.13)$$

which yields

$$\begin{aligned} \frac{\partial W}{\partial \lambda_1}\left(\lambda, \frac{1}{\lambda}, 1\right) &= \frac{d}{dt} W\left(\lambda + t, \frac{1}{\lambda}, 1\right)\Big|_{t=0} = \frac{d}{dt} W\left(\frac{1}{\lambda + t}, \lambda, 1\right)\Big|_{t=0} = \frac{d}{dt} W\left(\lambda, \frac{1}{\lambda + t}, 1\right)\Big|_{t=0} \\ &= \frac{\partial W}{\partial \lambda_2}\left(\lambda, \frac{1}{\lambda + t}, 1\right) \frac{-1}{(\lambda + t)^2}\Big|_{t=0} = -\frac{1}{\lambda^2} \frac{\partial W}{\partial \lambda_2}\left(\lambda, \frac{1}{\lambda}, 1\right) \end{aligned}$$

and

$$\begin{aligned} \frac{\partial W}{\partial \lambda_3}\left(\lambda, \frac{1}{\lambda}, 1\right) &= \frac{d}{dt} W\left(\lambda, \frac{1}{\lambda}, 1 + t\right)\Big|_{t=0} = \frac{d}{dt} W\left(\frac{1}{\lambda}, \lambda, \frac{1}{1 + t}\right)\Big|_{t=0} = \frac{d}{dt} W\left(\lambda, \frac{1}{\lambda}, \frac{1}{1 + t}\right)\Big|_{t=0} \\ &= \frac{\partial W}{\partial \lambda_3}\left(\lambda, \frac{1}{\lambda}, \frac{1}{1 + t}\right) \frac{-1}{(1 + t)^2}\Big|_{t=0} = -\frac{\partial W}{\partial \lambda_3}\left(\lambda, \frac{1}{\lambda}, 1\right), \end{aligned}$$

thus the conditions (4.12) are both satisfied in this case. \blacksquare

In terms of the principal invariants, the tension-compression symmetry of an energy W can also be expressed as $W(I_1, I_2, I_3) = W(\frac{I_2}{I_3}, \frac{I_1}{I_3}, \frac{1}{I_3})$. In the context of *anti-plane shear deformations* (cf. Appendix C), it was recently shown [40] that the so-called ‘‘condition K1 with $b = \frac{1}{2}$ ’’, which is exactly the equality $\frac{\partial W}{\partial I_1} = \frac{\partial W}{\partial I_2}$ given in eq. (4.8), is always satisfied by tension-compression symmetric energies.

Lemma 4.1 can be extended to include a volumetric term, yielding a more applicable sufficient criterion for the compatibility of shear deformations and shear stress.

Theorem 4.2. *Let $W: \text{GL}^+(3) \rightarrow \mathbb{R}$ be an elastic energy potential of the form*

$$W(F) = W_{\text{tc}}(F) + f(\det F), \quad (4.14)$$

where $W_{\text{tc}}: \text{GL}^+(3) \rightarrow \mathbb{R}$ is a sufficiently smooth tension-compression symmetric function and $f: \mathbb{R}^+ \rightarrow \mathbb{R}$ is differentiable with $f'(1) = 0$. Then $\hat{\sigma}(B) = \hat{\sigma}(V^2)$ is a pure shear stress for every finite pure shear stretch $V = V_\alpha$.

Proof. Since

$$\frac{\partial W}{\partial \lambda_i}(\lambda_1, \lambda_2, \lambda_3) = \frac{\partial W_{\text{tc}}}{\partial \lambda_i}(\lambda_1, \lambda_2, \lambda_3) + \frac{d}{d\lambda_i} f(\lambda_1 \lambda_2 \lambda_3) = \frac{\partial W_{\text{tc}}}{\partial \lambda_i}(\lambda_1, \lambda_2, \lambda_3) + \frac{\lambda_1 \lambda_2 \lambda_3}{\lambda_i} f'(\lambda_1 \lambda_2 \lambda_3),$$

we can apply Lemma 4.1 to W_{tc} and compute

$$\frac{\partial W}{\partial \lambda_1}\left(\lambda, \frac{1}{\lambda}, 1\right) = \frac{\partial W_{\text{tc}}}{\partial \lambda_1}\left(\lambda, \frac{1}{\lambda}, 1\right) + \underbrace{\frac{1}{\lambda} f'(1)}_{=0} = -\frac{1}{\lambda^2} \frac{\partial W_{\text{tc}}}{\partial \lambda_2}\left(\lambda, \frac{1}{\lambda}, 1\right) - \underbrace{\frac{1}{\lambda} f'(1)}_{=0} = -\frac{1}{\lambda^2} \frac{\partial W}{\partial \lambda_2}\left(\lambda, \frac{1}{\lambda}, 1\right)$$

as well as

$$\frac{\partial W}{\partial \lambda_3}\left(\lambda, \frac{1}{\lambda}, 1\right) = \frac{\partial W_{\text{tc}}}{\partial \lambda_3}\left(\lambda, \frac{1}{\lambda}, 1\right) + \underbrace{f'(1)}_{=0} = -\frac{\partial W_{\text{tc}}}{\partial \lambda_2}\left(\lambda, \frac{1}{\lambda}, 1\right) - \underbrace{f'(1)}_{=0} = -\frac{\partial W}{\partial \lambda_3}\left(\lambda, \frac{1}{\lambda}, 1\right)$$

for all $F \in \text{GL}^+(3)$ with singular values $\lambda_1, \lambda_2, \lambda_3$, thus both conditions (4.12) are satisfied under the theorem’s assumptions. \blacksquare

Example 4.3. For the important class of *Hencky type* isotropic elastic energy functions [21, 8, 9, 22, 23, 24, 20, 17]

$$W(F) = \mathcal{W}(\|\text{dev } \log V\|^2, |\text{tr } \log V|^2), \quad (4.15)$$

the requirement of tension-compression symmetry is always satisfied. Thus for every energy of the form (4.15), every finite pure shear stretch V_α corresponds to a Cauchy pure shear stress $\hat{\sigma}(B)$. These energies include the classical Hencky energy [9]

$$W_{\text{H}}(F) = \mu \|\text{dev}_3 \log U\|^2 + \frac{\kappa}{2} (\text{tr}(\log U))^2 = \mu \|\log U\|^2 + \frac{\Lambda}{2} (\text{tr}(\log U))^2 \quad (4.16)$$

as well as the so-called *exponentiated Hencky energy* [23]

$$W_{\text{eH}}(U) = \frac{\mu}{k} e^{k\|\text{dev } \log V\|^2} + \frac{\kappa}{2\widehat{k}} e^{\widehat{k}[\text{tr } \log V]^2}.$$

A particularly interesting application of Theorem 4.2 is the class of energy functions which exhibit an *additive isochoric-volumetric split*, i.e. energies of the form

$$W(F) = W_{\text{iso}}\left(\frac{F}{(\det F)^{1/3}}\right) + f(\det F).$$

In terms of the principal invariants, such energy functions can also be expressed as

$$W(I_1, I_2, I_3) = W_{\text{iso}}(I_1 I_3^{-\frac{1}{3}}, I_2 I_3^{-\frac{2}{3}}) + f(I_3).$$

Note that the condition $f'(1) = 0$ is necessarily satisfied for such energy functions if the reference configuration $F = \mathbb{1}$ is stress free.

Corollary 4.4. *Let W be an isotropic elastic energy with an additive isochoric-volumetric split and a stress-free reference configuration. If W_{iso} is tension-compression symmetric⁵, i.e. if $W_{\text{iso}}(J_1, J_2) = W_{\text{iso}}(J_2, J_1)$ for all $F \in \text{GL}^+(3)$, then $\hat{\sigma}(B)$ is a pure shear stress for every finite pure shear stretch $V = V_\alpha$.*

Example 4.5. As an example for the application of Corollary 4.4, consider a special format of a slightly compressible Mooney-Rivlin energy

$$W(I_1, I_2, I_3) = \frac{\mu}{4} \left((I_1 I_3^{-\frac{1}{3}} - 3) + (I_2 I_3^{-\frac{2}{3}} - 3) \right) + f(I_3), \quad (4.17)$$

consisting of the tension-compression symmetric isochoric term $W_{\text{iso}}(J_1, J_2) = \frac{\mu}{4} ((J_1 - 3) + (J_2 - 3))$ and arbitrary volumetric term $f(I_3)$ with $f'(1) = 0$. ■

Another important class of functions to (some of) which Theorem 4.2 is applicable are the so-called *Valanis-Landel energies* of the form

$$W(\lambda_1, \lambda_2, \lambda_3) = \sum_{i=1}^3 w(\lambda_i) + f(\det F)$$

with functions $w, f: \mathbb{R}^+ \rightarrow \mathbb{R}$. These energies were originally introduced by Valanis and Landel [38] as a model for incompressible materials, but are often coupled with a volumetric term and applied to the compressible case as well.

Corollary 4.6. *Let W be an isotropic elastic energy of the generalized Valanis-Landel form, i.e.*

$$W(F) = \sum_{i=1}^3 w(\lambda_i) + f(\det F) = w(\lambda_1) + w(\lambda_2) + w(\lambda_3) + f(\lambda_1 \lambda_2 \lambda_3) \quad (4.18)$$

for all $F \in \text{GL}^+(3)$ with singular values $\lambda_1, \lambda_2, \lambda_3$, where $w, f: \mathbb{R}^+ \rightarrow \mathbb{R}$ are differentiable functions. If the reference configuration is stress free and w is tension-compression symmetric, i.e. $w(\lambda) = w(\frac{1}{\lambda})$ for all $\lambda \in \mathbb{R}^+$, then $\hat{\sigma}(B)$ is a pure shear stress for every finite pure shear stretch $V = V_\alpha$.

⁵Note that tension-compression symmetry of $W(F)$ implies tension-compression symmetry of $W_{\text{iso}}(J_1, J_2)$ because $W(F) = W(F^{-1}) \iff W(\frac{I_2}{I_3}, \frac{I_1}{I_3}, \frac{1}{I_3}) \xrightarrow{I_3=1} W(I_1, I_2, 1) = W(I_2, I_1, 1) \iff W_{\text{iso}}(J_1, J_2) = W_{\text{iso}}(J_2, J_1)$.

Proof. It is easy to verify that a function $W: \text{GL}^+(3)$ with $W(F) = \sum_{i=1}^3 w(\lambda_i)$ for all F with singular values λ_i is tension-compression symmetric if and only if $w(\frac{1}{\lambda}) = w(\lambda)$ and that the requirement of a stress-free reference configuration implies $f'(1) = 0$, thus Theorem 4.2 is directly applicable. ■

The condition $w(\frac{1}{\lambda}) = w(\lambda)$ is satisfied for a large variety of Valanis-Landel type elastic energy potentials, including, for example, the *Bažant energy* [2]

$$W(F) = \frac{\mu}{4} \|B - B^{-1}\|^2 = \frac{\mu}{4} \sum_{i=1}^3 \left(\lambda_i^2 - \frac{1}{\lambda_i^2} \right)^2$$

or, again, the logarithmic-quadratic Hencky energy (4.16), which can be expressed in the form (4.18) with $w(\lambda) = \mu \log^2(\lambda)$ and $f(d) = \frac{\Delta}{2} (\log d)^2$.

Of course, without any additional conditions, a pure shear stretch does *not* necessarily induce a Cauchy pure shear stress for arbitrary constitutive laws.

Example 4.7. Consider the *Blatz-Ko* type energy [11]

$$W(F) = \frac{\mu}{2} \left(\|F\|^2 + \frac{2}{\det F} - 5 \right) = \frac{\mu}{2} \left(I_1 + \frac{2}{\sqrt{I_3}} - 5 \right) \quad (4.19)$$

with the corresponding Cauchy stress response

$$\hat{\sigma}(B) = \beta_0 \mathbb{1} + \beta_1 B = \frac{\mu}{\sqrt{I_3}} B - \frac{\mu}{I_3} \mathbb{1} = \frac{\mu}{I_3} \left(\sqrt{I_3} B - \mathbb{1} \right). \quad (4.20)$$

For a deformation of the form (4.1), we find

$$\hat{\sigma}(B) = \mu \text{diag}(\lambda - 1, \frac{1}{\lambda} - 1, 0).$$

Thus, a finite pure shear stretch V_α can only correspond to a Cauchy pure shear stress if

$$\lambda - 1 = 1 - \frac{1}{\lambda} \quad \iff \quad \lambda + \frac{1}{\lambda} = 2 \quad \iff \quad \lambda = 1,$$

i.e. only in the trivial case $V = \mathbb{1}$.

Furthermore, if σ is a (diagonal) Cauchy pure shear stress of the form $\sigma = \text{diag}(s, -s, 0)$, then the eigenvalues of the corresponding stretch $V = \text{diag}(\lambda_1, \lambda_2, \lambda_3)$ satisfy the equalities

$$\begin{aligned} \text{diag}(s, -s, 0) &= \frac{\mu}{I_3} \left(\sqrt{I_3} B - \mathbb{1} \right) = \frac{\mu}{I_3} \left(\sqrt{I_3} \text{diag}(\lambda_1^2, \lambda_2^2, \lambda_3^2) - \mathbb{1} \right) \\ \implies & \quad 0 = \sqrt{I_3} \lambda_3^2 - 1 \quad \text{and} \quad \sqrt{I_3} \lambda_1^2 - 1 = - \left(\sqrt{I_3} \lambda_2^2 - 1 \right) \\ \iff & \quad 1 = \sqrt{I_3} \lambda_3^2 \quad \text{and} \quad \sqrt{I_3} (\lambda_1^2 + \lambda_2^2) = 2 \end{aligned}$$

If the stretch is volume preserving ($I_3 = 1$), then the first equation yields $\lambda_3 = 1$ (planarity). Similarly, if the stretch is planar, then the first equation reads $I_3 = 1$, i.e. the stretch is volume preserving. Since both properties can only be satisfied simultaneously for $V = \mathbb{1}$, any non-trivial deformation corresponding to a Cauchy pure shear stress is *neither* volume-preserving *nor* planar. ■

4.2 Pure shear stretch induced by pure shear stress

We now return to the question whether (or, more precisely, under which conditions) pure shear Cauchy stress induces a pure shear stretch V_α . Note again that while Theorem 4.2 and the subsequent corollaries ensure that every pure shear stretch V_α induces a Cauchy pure shear stress, additional assumptions on the energy function

are required to ensure the reverse implication since the Cauchy stress response is generally not invertible. The following theorem uses the condition of *Hill's (strict) inequality* [10]

$$\langle \tau(V_1) - \tau(V_2), \log(V_1) - \log(V_2) \rangle > 0 \quad \text{for all } V_1, V_2 \in \text{Sym}^+(n), V_1 \neq V_2, \quad (4.21)$$

where $\tau(V) = \det(V) \cdot \sigma(V) \det(V) \cdot \widehat{\sigma}(V^2)$ denotes the *Kirchhoff stress* corresponding to the stretch V . For hyperelastic materials, this inequality is equivalent to the strict convexity of the mapping $X \mapsto W(\exp(X))$ on $\text{Sym}(n)$.

Theorem 4.8. *Let W be a sufficiently smooth isotropic elastic energy satisfying the conditions (4.12). Furthermore, assume that W is p -coercive for some $p \geq 1$, i.e. $W(F) \geq d + c \cdot \|F\|^p$ for some $d \in \mathbb{R}$ and $c > 0$, and that W satisfies Hill's (strict) inequality (4.21). Then $\sigma(V)$ is a pure shear stress tensor if and only if V is a pure shear stretch.*

Proof. By assumption, if V is a pure shear stretch, then $\sigma(V)$ is a pure shear stress. In this case, the corresponding Kirchhoff stress $\tau(V) = \det(V) \cdot \sigma(V)$ is a pure shear stress as well. For $\lambda > 0$, define $s^\tau(\lambda)$ by

$$\tau(\text{diag}(\lambda, \frac{1}{\lambda}, 1)) = (s^\tau(\lambda), -s^\tau(\lambda), 0).$$

We first show that $\lambda \mapsto s^\tau(\lambda)$ is a surjective mapping from $(0, \infty)$ to \mathbb{R} . Due to (4.10),

$$s^\tau(\lambda) = \lambda \cdot \frac{\partial W}{\partial \lambda_1} \left(\lambda, \frac{1}{\lambda}, 1 \right) \quad \text{and} \quad -s^\tau(\lambda) = \frac{1}{\lambda} \cdot \frac{\partial W}{\partial \lambda_2} \left(\lambda, \frac{1}{\lambda}, 1 \right)$$

and thus

$$\frac{d}{d\lambda} W \left(\lambda, \frac{1}{\lambda}, 1 \right) = \frac{\partial W}{\partial \lambda_1} \left(\lambda, \frac{1}{\lambda}, 1 \right) - \frac{1}{\lambda^2} \frac{\partial W}{\partial \lambda_2} \left(\lambda, \frac{1}{\lambda}, 1 \right) = \frac{2}{\lambda} s^\tau(\lambda).$$

Now, assume that $s^\tau(\lambda) \leq C$ for all $\lambda \in \mathbb{R}^+$ for some $C \in \mathbb{R}$. Then for all $\lambda_0 > 1$,

$$W \left(\lambda_0, \frac{1}{\lambda_0}, 1 \right) = W(1, 1, 1) + \int_1^{\lambda_0} \frac{d}{d\lambda} W \left(\lambda, \frac{1}{\lambda}, 1 \right) d\lambda \leq W(1, 1, 1) + \int_1^{\lambda_0} \frac{2C}{\lambda} d\lambda = W(1, 1, 1) + 2C \log(\lambda_0), \quad (4.22)$$

in contradiction to the coercivity of W , which implies

$$W \left(\lambda_0, \frac{1}{\lambda_0}, 1 \right) \geq d + \tilde{c} \cdot \left\| \text{diag} \left(\lambda_0, \frac{1}{\lambda_0}, 1 \right) \right\|_2^p = d + \tilde{c} \cdot \lambda_0^p$$

for some appropriate constant $\tilde{c} > 0$, where $\|\cdot\|_2$ denotes the spectral norm. Similarly, we find that $s^\tau(\lambda)$ cannot be bounded below for $\lambda \rightarrow 0$. In particular, the mapping $\lambda \mapsto s^\tau(\lambda)$ from $(0, \infty)$ to \mathbb{R} is surjective.

Now let $V \in \text{Sym}^+(n)$ such that for some $s \in \mathbb{R}$

$$\sigma(V) = \begin{pmatrix} 0 & s & 0 \\ s & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}, \quad \text{i.e.} \quad \tau(V) = \begin{pmatrix} 0 & s^\tau & 0 \\ s^\tau & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \text{with } s^\tau = \frac{s}{\det V}.$$

Choose $\lambda > 0$ such that $s^\tau = s^\tau(\lambda)$. Then

$$\tau(V) = Q \text{diag}(s^\tau, -s^\tau, 0) Q^T = Q \tau(\text{diag}(\lambda, \frac{1}{\lambda}, 1)) Q^T = \tau(Q \text{diag}(\lambda, \frac{1}{\lambda}, 1) Q) \quad (4.23)$$

with $Q \in \text{SO}(3)$ given by (2.3). Since Hill's strict inequality implies that the mapping $\log V \mapsto \tau(V)$ is strictly monotone and hence injective, the mapping $V \mapsto \tau(V)$ is injective as well. Therefore, (4.23) implies that $V = Q \text{diag}(\lambda, \frac{1}{\lambda}, 1) Q$ is a pure shear stretch. \blacksquare

Remark 4.9. Note that in the proof of Proposition 4.8, the coercivity of the energy is used only to contradict inequality (4.22). It is easy to see that the condition of p -coercivity can be replaced by the weaker requirement that W has “stronger-than-logarithmic” growth, i.e. $\log(\|F\|) \in o(W(\|F\|))$. In particular, the result is also applicable to the classical quadratic-logarithmic *Hencky energy* (4.16) which is not p -coercive for any $p \geq 1$, but does satisfy $W(F) \geq c \log^2(\|F\|)$ for some appropriate $c > 0$.⁶

Remark 4.10. In particular, the equalities (4.12) required in Theorem 4.8 hold if the conditions of Theorem 4.2 are satisfied.

Conclusion

While the incompatibility between simple shear and Cauchy pure shear stress described by Destrade et al. [6] and Moon and Truesdell [18] as well as Mihai and Goriely [15] is due to the difference between the *principal axes* of strain and stress, i.e. the eigenspaces of $B = F_\gamma F_\gamma^T$ and $\sigma = \sigma^s$, the question whether or not a pure shear Cauchy stress corresponds to a pure shear stretch V_α depends only on the *principal stretches* and *principal stresses*, i.e. the eigenvalues of B and σ . In particular, this latter question of compatibility is a matter of the constitutive law and depends on the specific choice of a stress response function. Physical reasons suggest that a correspondence between pure shear stretch and pure shear stress is plausible, as further demonstrated by the criteria and examples considered here. As a substitute for the concept of simple shear in finite elasticity, we introduced the notion of an idealized “finite simple shear” deformation (gradient) which not only corresponds to a pure shear stretch tensor, but is also coaxial to pure shear stress tensors.

In order to avoid the confusion arising from the apparent similarities to the linear case, we suggest to use the term “*simple glide*” instead of “simple shear” for deformations F_γ in finite elasticity.

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5 References

- [1] M. Baker and J. Ericksen. “Inequalities restricting the form of the stress-deformation relations for isotropic elastic solids and Reiner-Rivlin fluids”. *Journal of the Washington Academy of Sciences* 44.2 (1954). Pp. 33–35.
- [2] Z. P. Bažant. “Easy-to-compute tensors with symmetric inverse approximating Hencky finite strain and its rate”. *Journal of Engineering Materials and Technology* 120.2 (1998). Pp. 131–136.
- [3] G. F. Becker. “The finite elastic stress-strain function”. *American Journal of Science* 46 (1893). Pp. 337–356 newly typeset version available at https://www.uni-due.de/imperia/md/content/mathematik/ag_neff/becker_latex_new1893.pdf.
- [4] P. Belk and R. Fosdick. “The state of pure shear”. *Journal of Elasticity* 52.1 (1998). Pp. 91–98.
- [5] P. Boulanger and M. Hayes. “On pure shear”. *Journal of Elasticity* 77.1 (2004). Pp. 83–89.
- [6] M. Destrade, J. G. Murphy, and G. Saccomandi. “Simple shear is not so simple”. *International Journal of Non-Linear Mechanics* 47.2 (2012). Pp. 210–214. issn: 0020-7462.
- [7] J. E. Dunn. “Elastic materials of coaxial type and inequalities sufficient to ensure strong ellipticity”. *International Journal of Solids and Structures* 20.5 (1984). Pp. 417–427.
- [8] H. Hencky. “Über die Form des Elastizitätsgesetzes bei ideal elastischen Stoffen”. *Zeitschrift für technische Physik* 9 (1928). Pp. 215–220 available at www.uni-due.de/imperia/md/content/mathematik/ag_neff/hencky1928.pdf.
- [9] H. Hencky. “Welche Umstände bedingen die Verfestigung bei der bildsamen Verformung von festen isotropen Körpern?”. *Zeitschrift für Physik* 55 (1929). Pp. 145–155 available at www.uni-due.de/imperia/md/content/mathematik/ag_neff/hencky1929.pdf.

⁶The correspondence between pure shear stretch and pure shear Cauchy stress for the classical Hencky model was shown earlier by Vallée [39].

- [10] R. Hill. “Constitutive inequalities for isotropic elastic solids under finite strain”. *Proceedings of the Royal Society of London. A. Mathematical and Physical Sciences* 314 (1970). Pp. 457–472.
- [11] C. Horgan. “Remarks on ellipticity for the generalized Blatz-Ko constitutive model for a compressible nonlinearly elastic solid”. *Journal of Elasticity* 42.2 (1996). Pp. 165–176.
- [12] C. Horgan and J. Murphy. “Simple shearing of incompressible and slightly compressible isotropic nonlinearly elastic materials”. *Journal of Elasticity* 98.2 (2010). Pp. 205–221.
- [13] T. Lubensky. *Nonlinear Elasticity: Lectures at the Princeton Center for Complex Materials Summer School on Condensed Matter Physics*. July 2005. URL: http://www.physics.upenn.edu/~7Etom/talks/Nonlinear_Elasticity.pdf.
- [14] A. I. Lurie. *Nonlinear Theory of Elasticity*. Elsevier, 2012.
- [15] L. A. Mihai and A. Goriely. “Positive or negative Poynting effect? The role of adscititious inequalities in hyperelastic materials”. *Proc. R. Soc. A* 467.2136 (2011). Pp. 3633–3646.
- [16] L. A. Mihai and A. Goriely. “How to characterize a nonlinear elastic material? A review on nonlinear constitutive parameters in isotropic finite elasticity”. *Proc. R. Soc. A* 473.2207 (2017). P. 20170607.
- [17] G. Montella, S. Govindjee, and P. Neff. “The exponentiated Hencky strain energy in modeling tire derived material for moderately large deformations”. *Journal of Engineering Materials and Technology* 138.3 (2016). Pp. 031008-1–031008-12.
- [18] H. Moon and C. Truesdell. “Interpretation of adscititious inequalities through the effects pure shear produces upon an isotropic elastic solid”. *Archive for Rational Mechanics and Analysis* 55.1 (1974). Pp. 1–17.
- [19] D. Moreira and L. Nunes. “Comparison of simple and pure shear for an incompressible isotropic hyperelastic material under large deformation”. *Polymer Testing* 32.2 (2013). Pp. 240–248.
- [20] B. Nedjar, H. Baaser, R. J. Martin, and P. Neff. “A finite element implementation of the isotropic exponentiated Hencky-logarithmic model and simulation of the eversion of elastic tubes”. *Computational Mechanics* (2017). Pp. 1–20.
- [21] P. Neff, B. Eidel, and R. J. Martin. “The axiomatic deduction of the quadratic Hencky strain energy by Heinrich Hencky”. *arXiv:1402.4027* (2014).
- [22] P. Neff, B. Eidel, and R. J. Martin. “Geometry of logarithmic strain measures in solid mechanics”. *Archive for Rational Mechanics and Analysis* 222.2 (2016). Pp. 507–572 available at arXiv:1505.02203. DOI: 10.1007/s00205-016-1007-x.
- [23] P. Neff, I.-D. Ghiba, and J. Lankeit. “The exponentiated Hencky-logarithmic strain energy. Part I: Constitutive issues and rank-one convexity”. *Journal of Elasticity* 121.2 (2015). Pp. 143–234. DOI: 10.1007/s10659-015-9524-7.
- [24] P. Neff, J. Lankeit, I.-D. Ghiba, R. J. Martin, and D. J. Steigmann. “The exponentiated Hencky-logarithmic strain energy. Part II: coercivity, planar polyconvexity and existence of minimizers”. *Zeitschrift für angewandte Mathematik und Physik* 66.4 (2015). Pp. 1671–1693. DOI: 10.1007/s00033-015-0495-0.
- [25] P. Neff, I. Münch, and R. J. Martin. “Rediscovering G. F. Becker’s early axiomatic deduction of a multiaxial nonlinear stress–strain relation based on logarithmic strain”. *Mathematics and Mechanics of Solids* 21.7 (2016). Pp. 856–911. DOI: 10.1177/1081286514542296.
- [26] L. Nunes and D. Moreira. “Simple shear under large deformation: experimental and theoretical analyses”. *European Journal of Mechanics-A/Solids* 42 (2013). Pp. 315–322.
- [27] R. W. Ogden. *Non-Linear Elastic Deformations*. 1st ed. Mathematics and its Applications. Chichester: Ellis Horwood, 1983.
- [28] P. J. Olver. *Equivalence, invariants and symmetry*. Cambridge University Press, 1995.
- [29] J. H. Poynting. “On pressure perpendicular to the shear planes in finite pure shears, and on the lengthening of loaded wires when twisted”. *Proc. R. Soc. Lond. A* 82.557 (1909). Pp. 546–559.
- [30] H. Richter. “Das isotrope Elastizitätsgesetz”. *Zeitschrift für Angewandte Mathematik und Mechanik* 28.7/8 (1948). Pp. 205–209 available at https://www.uni-due.de/imperia/md/content/mathematik/ag_neff/richter_isotrop_log.pdf.
- [31] C. Thiel. “Neue Matrix-Ungleichungen und Anwendungen auf konstitutive Beziehungen in der nichtlinearen Elastizitätstheorie”. PhD thesis. Universitätsbibliothek Duisburg-Essen, 2017.
- [32] C. Thiel, J. Voss, R. J. Martin, and P. Neff. “Do we need Truesdell’s empirical inequalities? On the coaxiality of stress and stretch.” *in preparation* (2018).
- [33] C. Truesdell. “Mechanical foundations of elasticity and fluid dynamics”. *Journal of Rational Mechanics and Analysis* 1 (1952). Pp. 125–300.
- [34] C. Truesdell. “Das ungelöste Hauptproblem der endlichen Elastizitätstheorie”. *Zeitschrift für Angewandte Mathematik und Mechanik* 36.3-4 (1956). Pp. 97–103.
- [35] C. Truesdell and H. Moon. “Inequalities sufficient to ensure semi-invertibility of isotropic functions”. *Journal of Elasticity* 5.34 (1975).
- [36] C. Truesdell and W. Noll. “The Non-Linear Field Theories of Mechanics”. *Handbuch der Physik*. Ed. by S. Flügge. Vol. III/3. Heidelberg: Springer, 1965.
- [37] C. Truesdell and R. Toupin. “Static grounds for inequalities in finite strain of elastic materials”. *Archive for Rational Mechanics and Analysis* 12.1 (1963). Pp. 1–33.

- [38] K. C. Valanis and R. F. Landel. “The strain-energy Function of a hyperelastic material in terms of the extension ratios”. *Journal of Applied Physics* 38.7 (1967). Pp. 2997–3002.
- [39] C. Vallée. “Lois de comportement élastique isotropes en grandes déformations”. *International Journal of Engineering Science* 16.7 (1978). Pp. 451–457.
- [40] J. Voss, H. Baaser, R. J. Martin, and P. Neff. “Again anti-plane shear”. *to appear in International Journal of Non-Linear Mechanics* (2018). available at arXiv:1803.00265.

A Biot pure shear stress

The following lemmas and theorems discuss the connection between Biot pure shear stress T^{Biot} and right finite simple shear deformations F_α analogues to Cauchy pure shear stress. For each of the results presented here, we also provide the reference to the respective counterpart for the Cauchy stress tensor.

Proposition A.1 (Proposition 2.8). *Let $C \in \text{Sym}^+(3)$ be given by (2.4) with $p > |q|$ and $r > 0$. Then $F \in \text{GL}^+(3)$ with $F^T F = C$ is uniquely determined by*

$$F = Q \text{diag}(a, b, c) F_\gamma = Q \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad (\text{A.1})$$

up to an arbitrary $Q \in \text{SO}(3)$, where

$$a = \sqrt{p}, \quad b = \sqrt{\frac{p^2 - q^2}{p}}, \quad c = \sqrt{r} \quad \text{and} \quad \gamma = \frac{q}{p} \quad (\text{A.2})$$

or, in terms of the singular values $\lambda_1, \lambda_2, \lambda_3 \in \mathbb{R}_+$ of F ,

$$a = \sqrt{\frac{\lambda_1^2 + \lambda_2^2}{2}}, \quad b = \lambda_1 \lambda_2 \sqrt{\frac{2}{\lambda_1^2 + \lambda_2^2}}, \quad c = \lambda_3 \quad \text{and} \quad \gamma = \frac{\lambda_1^2 - \lambda_2^2}{\lambda_1^2 + \lambda_2^2}. \quad (\text{A.3})$$

In particular, F is necessarily of the form (A.1) if the Biot stress tensor corresponding to the deformation gradient F induced by an isotropic law of Cauchy elasticity is a pure shear stress of the form $T^{\text{Biot}} = s(e_1 \otimes e_2 + e_2 \otimes e_1)$ with $s \in \mathbb{R}$.

Proof. Let $\tilde{F} = F_\gamma \text{diag}(a, b, c)$ with a, b, c given by (2.10). Then $a^2 + b^2 \gamma^2 = b^2 = p$, $b^2 \gamma = q$ and $c^2 = r$, thus

$$\tilde{F}^T \tilde{F} = \begin{pmatrix} 1 & 0 & 0 \\ \gamma & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} = \begin{pmatrix} a^2 & a^2 \gamma & 0 \\ a^2 \gamma & a^2 \gamma^2 + b^2 & 0 \\ 0 & 0 & c^2 \end{pmatrix} = \begin{pmatrix} p & q & 0 \\ q & p & 0 \\ 0 & 0 & r \end{pmatrix}.$$

Due to Lemma 2.5, $p = \frac{1}{2}(\lambda_1^2 + \lambda_2^2)$, $q = \frac{1}{2}(\lambda_1^2 - \lambda_2^2)$ and $r = \lambda_3^2$, which immediately implies $a = \sqrt{\frac{\lambda_1^2 + \lambda_2^2}{2}}$, $c = \lambda_3$ and $\gamma = \frac{\lambda_1^2 - \lambda_2^2}{\lambda_1^2 + \lambda_2^2}$. Moreover, $p + q = \lambda_1^2$ and $p - q = \lambda_2^2$, thus $p^2 - q^2 = \lambda_1^2 \lambda_2^2$ and therefore $b = \lambda_1 \lambda_2 \sqrt{\frac{2}{\lambda_1^2 + \lambda_2^2}}$.

Now let B be given by (2.4) and consider an arbitrary $F \in \text{GL}^+(3)$ with $F^T F = C = \tilde{F}^T \tilde{F}$. Since F is uniquely determined by $F^T F$ up to a left-hand rotation, there exists $Q \in \text{SO}(3)$ with $F = Q \tilde{F}$, thus F is of the form (A.1). \blacksquare

Lemma A.2 (Lemma 3.5). *Let the stretch $C = P$ be of the form (3.1). Then the deformation gradient $F \in \text{GL}^+(3)$ with $F^T F = C$ is of the form*

$$F = Q \cdot \begin{pmatrix} \sqrt{\cosh(2\alpha)} & \frac{\sinh(2\alpha)}{\sqrt{\cosh(2\alpha)}} & 0 \\ 0 & \frac{1}{\sqrt{\cosh(2\alpha)}} & 0 \\ 0 & \frac{\sqrt{\cosh(2\alpha)}}{0} & 1 \end{pmatrix} = Q \cdot \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(2\alpha) & \sinh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix}. \quad (\text{A.4})$$

with arbitrary $Q \in \text{SO}(3)$.

Proof. We use Proposition A.1 and write $\lambda_1 = e^\alpha$, $\lambda_2 = e^{-\alpha}$, $\lambda_3 = 1$. Then

$$\begin{aligned} \gamma &= \frac{\lambda_1^2 - \lambda_2^2}{\lambda_1^2 + \lambda_2^2} = \frac{e^{2\alpha} - e^{-2\alpha}}{e^{2\alpha} + e^{-2\alpha}} = \tanh(2\alpha), & a &= \sqrt{\frac{\lambda_1^2 + \lambda_2^2}{2}} = \sqrt{\frac{e^{2\alpha} + e^{-2\alpha}}{2}} = \sqrt{\cosh(2\alpha)}, \\ b &= \lambda_1 \lambda_2 \sqrt{\frac{2}{\lambda_1^2 + \lambda_2^2}} = e^\alpha e^{-\alpha} \cdot \sqrt{\frac{2}{e^{2\alpha} + e^{-2\alpha}}} = \frac{1}{\sqrt{\cosh(2\alpha)}} & \text{and} & \quad c = \lambda_3 = 1 \end{aligned}$$

and thus

$$\begin{aligned}
F &= Q \begin{pmatrix} a & 0 & 0 \\ 0 & b & 0 \\ 0 & 0 & c \end{pmatrix} \begin{pmatrix} 1 & \gamma & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} = Q \begin{pmatrix} \sqrt{\cosh(2\alpha)} & 0 & 0 \\ 0 & \frac{1}{\sqrt{\cosh(2\alpha)}} & 0 \\ 0 & 0 & 1 \end{pmatrix} \begin{pmatrix} 1 & \tanh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \\
&= Q \cdot \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(2\alpha) & \sinh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix}.
\end{aligned}$$

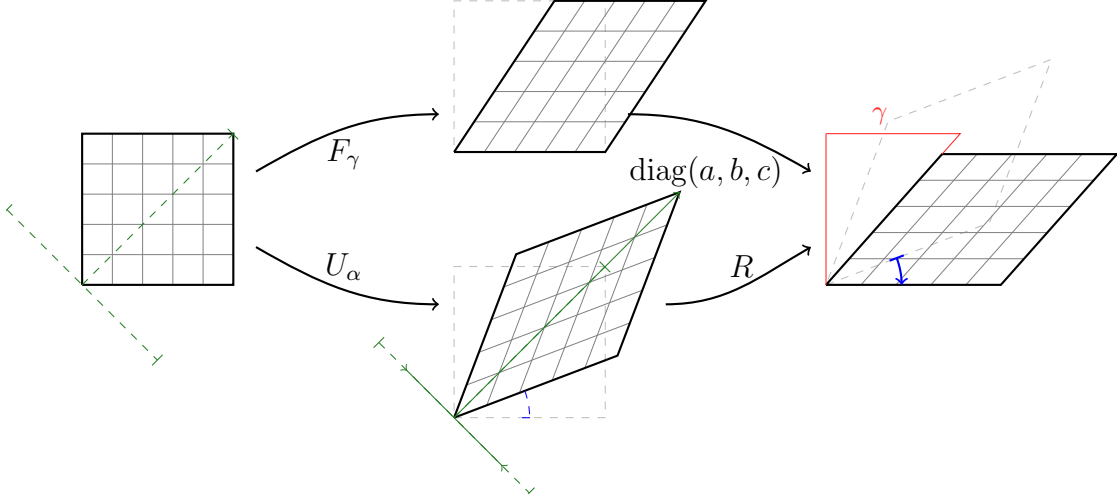


Figure 10: Right finite simple shear deformation with amount of shear $\gamma = \sinh(2\alpha)$.

Using Lemma A.1, we can now prove Theorem 3.7.

Proof of Theorem 3.7. Let T^{Biot} be a pure shear stress tensor with $s \neq 0$ and $C \in \text{Sym}^+(3)$ with $\lambda_1 = \lambda$, $\lambda_2 = \frac{1}{\lambda}$, $\lambda_3 = 1$ for arbitrary $\lambda \in \mathbb{R}_+$ with $\lambda_1^2, \lambda_2^2, \lambda_3^2$ as the eigenvalues of C . By Lemma 3.3, the tensors C and $\hat{T}^{\text{Biot}}(C)$ commute if and only if $C = U^2$ is of the form (3.1), i.e. if and only if

$$U = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix} = \exp \begin{pmatrix} 0 & \alpha & 0 \\ \alpha & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} \quad \text{with } \alpha \in \mathbb{R} \quad (\text{A.5})$$

is a finite pure shear stretch.

For an isotropic elasticity law the right Cauchy-Green deformation tensors $C = F^T F$ commutes with $\hat{T}^{\text{Biot}}(C)$. Thus if $\hat{T}^{\text{Biot}}(C)$ is a Biot pure shear stress, then $C = F^T F$ is of the form (3.1) (i.e. U is a finite pure shear stretch). By Lemma A.2, the deformation gradient F is determined up to an arbitrary right-hand side rotation $Q \in \text{SO}(3)$, and it remains to observe that exactly for $Q = \mathbb{1}$, the resulting deformation F_α satisfies the condition of ground parallelism (i.e. has eigenvectors e_1 and e_3). ■

Remark A.3. Since in isotropic elasticity, the *second Piola-Kirchhoff stress tensor* S^2 commutes with the right Cauchy-Green deformation tensor $C = F^T F$ as well, the above results still hold if Biot stress T^{Biot} is replaced by S^2 .

It might also be of interest to investigate conditions on the energy W which ensure that the Biot stress T^{Biot} corresponding to a finite pure shear stretch U_α is a pure shear stress. Here we state, without proof, a result for hyperelastic models with energy functions of Valanis-Landel type.

Proposition A.4 (Corollary 4.6). *Let W be an isotropic elastic energy of the generalized Valanis-Landel form, i.e.*

$$W(F) = \sum_{i=1}^3 w(\lambda_i) + f(\det F) = w(\lambda_1) + w(\lambda_2) + w(\lambda_3) + f(\lambda_1 \lambda_2 \lambda_3)$$

for all $F \in \text{GL}^+(3)$ with singular values $\lambda_1, \lambda_2, \lambda_3$, where $w, f: \mathbb{R}^+ \rightarrow \mathbb{R}$ are differentiable functions. If the reference configuration is stress free and w satisfies $w'(\frac{1}{\lambda}) = -w'(\lambda)$ for all $\lambda \in \mathbb{R}^+$, then $\hat{T}^{\text{Biot}}(C) = \hat{T}^{\text{Biot}}(U^2)$ is a pure shear stress for every finite pure shear stretch $U = U_\alpha$.

For example, the requirement $w'(\frac{1}{\lambda}) = -w'(\lambda)$ from Proposition A.4 is satisfied by the function w originally proposed by Valanis and Landel as a model for incompressible materials [38, eq. (48)], which they defined via the equality $w'(\lambda) = 2\mu \log(\lambda)$, i.e. (assuming $W(\mathbf{1}) = 0$)

$$w(t) = 2\mu (t (\log(t) - 1) + 1),$$

which corresponds to the energy

$$W(U) = 2\mu \langle U, \log U - \mathbf{1} \rangle + 3. \quad (\text{A.6})$$

If this energy function is applied in the compressible case without an additional volumetric term, it induces *Becker's law of elasticity* [25] in the lateral contraction free case (zero Poisson's ratio). This constitutive law was first described by the geologist G.F. Becker [3], who deduced it from a number of axioms including the equivalence of finite pure shear stretch U and Biot pure shear stress; note that for Becker's stress-strain relation

$$T^{\text{Biot}}(U) = 2\mu \log U + \lambda \text{tr}(\log U) \mathbf{1}, \quad (\text{A.7})$$

every finite pure shear stretch $U = U_\alpha$ corresponds to a Biot pure shear stress and vice versa (cf. [39]), even in the non-hyperelastic case $\lambda \neq 0$. In fact, this compatibility property was one of Becker's major motivations for introducing his logarithmic constitutive model [3, 25].

B Linearization of finite simple shear

In order to verify that the notions of finite simple shear and finite pure shear stretch (as well as the rotation R given in (3.6)) are compatible to the corresponding concepts of shear in linear elasticity via the identification $\gamma = 2\alpha$, simply compare the linearizations

$$\begin{aligned} \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} 1 & \sinh(2\alpha) & 0 \\ 0 & \cosh(2\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} &= \begin{pmatrix} 1 & 2\alpha & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} + \mathcal{O}(\alpha^2), \\ \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(2\alpha) & \sinh(2\alpha) & 0 \\ 0 & 1 & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} &= \begin{pmatrix} 1 & 2\alpha & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} + \mathcal{O}(\alpha^2), \\ V_\alpha = \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ \sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & 1 \end{pmatrix} &= \mathbf{1} + \begin{pmatrix} 0 & \alpha & 0 \\ \alpha & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} + \mathcal{O}(\alpha^2) \end{aligned}$$

and

$$R = \frac{1}{\sqrt{\cosh(2\alpha)}} \begin{pmatrix} \cosh(\alpha) & \sinh(\alpha) & 0 \\ -\sinh(\alpha) & \cosh(\alpha) & 0 \\ 0 & 0 & \sqrt{\cosh(2\alpha)} \end{pmatrix} = \mathbf{1} + \begin{pmatrix} 0 & \alpha & 0 \\ -\alpha & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix} + \mathcal{O}(\alpha^2)$$

to Definitions 1.1 and 1.5.

C Shear monotonicity

Although simple shear is generally not a suitable concept for nonlinear elasticity if (Cauchy) pure shear stresses are concerned, it does occur in finite deformations under certain displacement boundary conditions in the context of so-called *anti-plane shear deformations* of the form $\varphi(x_1, x_2, x_3) = (x_1, x_2, x_3 + u(x_1, x_2))$. In the following, we discuss the concept of *shear monotonicity*, which has been the subject of a recent contribution by Voss et al. [40].

Recall from Remark 2.7 that for a simple shear $F = F_\gamma$, the left Cauchy-Green tensor $B = FF^T$ is given by

$$B = F_\gamma F_\gamma^T = \begin{pmatrix} 1 + \gamma^2 & \gamma & 0 \\ \gamma & 1 & 0 \\ 0 & 0 & 1 \end{pmatrix} \quad \text{and thus} \quad B^{-1} = \begin{pmatrix} 1 & -\gamma & 0 \\ -\gamma & 1 + \gamma^2 & 0 \\ 0 & 0 & 1 \end{pmatrix}, \quad (\text{C.1})$$

and from (4.2) that in the isotropic case, the Cauchy stress tensor can always be expressed in the form

$$\sigma = \beta_0 \mathbf{1} + \beta_1 B + \beta_{-1} B^{-1} \quad (\text{C.2})$$

with scalar-valued functions $\beta_0, \beta_1, \beta_{-1}$ depending on the matrix invariants I_1, I_2, I_3 of the stretch tensor $B = FF^T$. In the case (C.1) of simple shear, we find

$$I_1 = \text{tr} B = 3 + \gamma^2, \quad I_2 = \text{tr}(\text{Cof} B) = \text{tr} \begin{pmatrix} 1 & -\gamma & 0 \\ -\gamma & 1 + \gamma^2 & 0 \\ 0 & 0 & 1 \end{pmatrix} = 3 + \gamma^2, \quad I_3 = \det B = 1. \quad (\text{C.3})$$

This allows us to consider β_i as scalar-valued functions depending only on the amount of shear $\gamma \in \mathbb{R}^+$. The Cauchy stress tensor σ corresponding to a simple shear deformation F_γ with the amount of shear $\gamma \in \mathbb{R}$ is given by

$$\sigma = (\beta_0 + \beta_1 + \beta_{-1}) \mathbf{1} + \begin{pmatrix} \beta_1 \gamma^2 & (\beta_1 - \beta_{-1}) \gamma & 0 \\ (\beta_1 - \beta_{-1}) \gamma & \beta_{-1} \gamma^2 & 0 \\ 0 & 0 & 0 \end{pmatrix}. \quad (\text{C.4})$$

Again, we can observe that a simple shear deformation does not map to a non-trivial Cauchy pure shear stress, since the Cauchy stress tensor of a simple shear deformation is a pure shear stress if and only if

$$\begin{aligned} \beta_0 + (1 + \gamma^2)\beta_1 + \beta_{-1} &= 0, \\ \beta_0 + \beta_1 + (1 + \gamma^2)\beta_{-1} &= 0, \\ \beta_0 + \beta_1 + \beta_{-1} &= 0, \end{aligned} \quad \implies \quad \begin{aligned} \gamma^2(\beta_1 - \beta_{-1}) &= 0, \\ \beta_0 + \beta_1 + \beta_{-1} &= 0, \end{aligned} \quad (\text{C.5})$$

and for $\gamma \neq 0$, the equality $\gamma^2(\beta_1 - \beta_{-1}) = 0$ is satisfied if and only if $\beta_1 - \beta_{-1} = 0$, which results in an amount of shear stress $s = \gamma(\beta_1 - \beta_{-1}) = 0$.

Now, consider the entry $\sigma_{12}(\gamma) = (\beta_1 - \beta_{-1})\gamma$ as the amount of stress in e_1 -direction acting on a surface perpendicular to e_2 . From a physical point of view, increasing the amount of shear γ should clearly lead to an increase in the amount of Cauchy shear stress; this material property is also known as shear monotonicity [40]. Since

$$\sigma_{12}(\gamma) = (\beta_1 - \beta_{-1})\gamma = 2\gamma \left(\frac{\partial W}{\partial I_1} + \frac{\partial W}{\partial I_2} \right) \Big|_{I_1=I_2=3+\gamma^2, I_3=1} = \frac{d}{d\gamma} W(3 + \gamma^2, 3 + \gamma^2, 1), \quad (\text{C.6})$$

the condition for shear-monotonicity is given by

$$\frac{d}{d\gamma} \sigma_{12}(\gamma) = \frac{d^2}{(d\gamma)^2} W(3 + \gamma^2, 3 + \gamma^2, 1) > 0 \quad \forall \gamma \geq 0, \quad (\text{C.7})$$

which is equivalent to the convexity of the mapping $\gamma \mapsto W(3 + \gamma^2, 3 + \gamma^2, 1)$. This convexity property, known as *APS-convexity*, is equivalent to the convexity of the restriction of the energy functional $W(F)$ to the set of APS-deformations (i.e. deformations φ of the form $\varphi = (x_1, x_2, x_3 + u(x_1, x_2))$). In particular, APS-convexity is implied by rank-one convexity [40], thus every Cauchy stress response induced by a rank-one convex energy is shear monotone.